MERRIMACK RIVER BASIN TILTON, NEW HAMPSHIRE

### TILTON TOWN DAM NH 00151

**STATE NO 237.02** 

# PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM

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DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASS. 02154

JULY 1979

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20. ABSTRACT (Continue on reverse side II necessary and identify by block number)

The dam is 192 ft. long with a hydraulic height of 13 ft. The dam is in poor condition. Major concern is the poor condition of the timber frame spillway and wooden decking. It is small in size with a significant hazard potential.

# R R

#### DEPARTMENT OF THE ARMY

# NEW ENGLAND DIVISION, CORPS OF ENGINEERS 424 TRAPELO ROAD WALTHAM, MASSACHUSETTS 02154

REPLY TO ATTENTION OF:

NEDED

FEB 14 1930

Honorable Hugh J. Gallen Governor of the State of New Hampshire State House Concord, New Hampshire 03301

Dear Governor Gallen:

Inclosed is a copy of the Tilton Town Dam Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Water Resources Board, the cooperating agency for the State of New Hampshire. In addition, a copy of the report has also been furnished the owner, Town of Tilton.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Water Resources Board for your cooperation in carrying out this program.

Sincerely.

Incl As stated

MAX B. SCHEIDER

Colonel, Corps of Engineers

Division Engineer

#### NATIONAL DAM INSPECTION PROGRAM PHASE I INSPECTION REPORT

NH00151 Identification No.:

Name of Dam: Tilton Town Dam

Tilton Town:

Belknap, New Hampshire County and State: River:

Winnipesaukee River

April 6, 1979 and April 24, 1979 Date of Inspection:

#### BRIEF ASSESSMENT

Tilton Town Dam has a hydraulic height of 13 feet and a total length of 192 feet. It is a low, run-of-the-river dam and consists of a timber frame spillway with wooden upstream decking placed between two concrete sluiceways each controlled by a timber gate. The dam spans a reach of the Winnipesaukee River and is located in south central New Hampshire. The drainage area to the site consists of 473 square miles and includes the 363 square mile Lake Winnipesaukee drainage area. Maximum storage capacity is about 50 acre-feet. Tilton Town Dam is used to provide pondage for process water and water for an auxiliary fire The pond at spillway crest is 1450 feet in length with a surface area of about 4 acres.

The dam is in poor condition. Major concern is the poor condition of the timber frame spillway and wooden decking.

Based on small size and significant hazard classification in accordance with Corps guidelines, the test flood is 1/2 the Probable Maximum Flood (PMF). A test flood discharge of 7,570 cfs (16 csm) would overtop the dam by about 2.6 feet (5.6 feet over spillway crest) assuming both gates closed. The spillway will pass 2200 cfs or about 29 percent of the test flood. A major breach at top of dam would probably result in the loss of a few lives and could cause appreciable property damage.

The owner, the Town of Tilton, should implement the results of the recommendation and remedial measures given in Sections 7.2 and 7.3 or alternative in Section 7.4 within one year after receipt of this Phase I inspection report.

> Warren A. Guinan Project Manager N.H. P.E. 2339

This Phase I Inspection Report on Tilton Town Dam has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

Joseph a. Mc Elroy

JOSEPH A. MCELROY, MEMBER Foundation & Materials Branch Engineering Division

CARNEY M. TERZIAN, MEMBER

Design Branch

Engineering Division

JOSEPH FINEGAN, JR., CHAIRIAN

Chief, Reservoir Control Center

Water Control Branch Engineering Division

APPROVAL RECOMMENDED:

JOE B. FRYAR

Chief, Engineering Division

#### PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

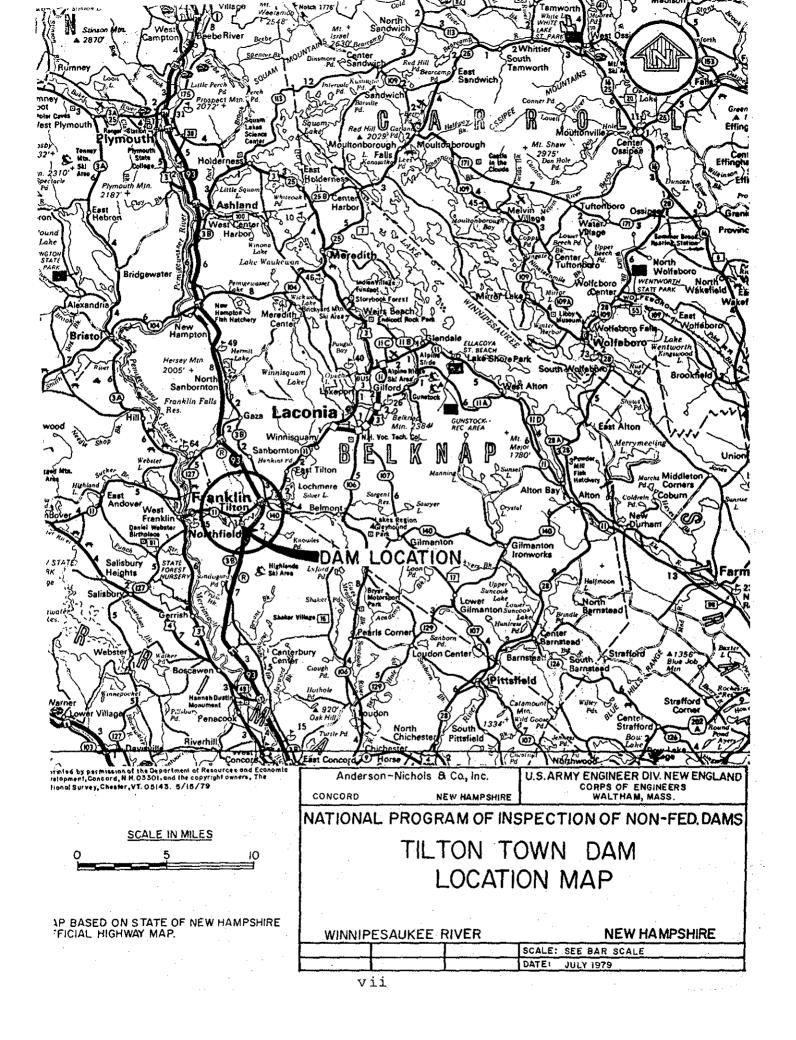
Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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APRIL 1979 Figure 1 - Overview of Tilton Town Dam.



# NATIONAL DAM INSPECTION PROGRAM PHASE I INSPECTION REPORT TILTON TOWN DAM

### SECTION 1 PROJECT INFORMATION

#### 1.1 General

a. Authority. Public Law 92-367, August 8, 1972 authorized the Secretary of the Army, through the Corps of Engineers, to initiate a National Program of Dam Inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Anderson-Nichols & Company, Inc. has been retained by the New England Division to inspect and report on selected dams in the State of New Hampshire. Authorization and notice to proceed were issued to Anderson-Nichols under a letter of November 20, 1978 from Max B. Scheider, Colonel, Corps of Engineers. Contract No. DACW33-79-C-0009 has been assigned by the Corps of Engineers for this work.

#### b. Purpose

- (1) To perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.
- (2) To encourage and prepare the States to initiate quickly effective dam safety programs for non-Federal dams.
- (3) To update, verify and complete the National Inventory of Dams.

#### 1.2 Description of Project

a. Location. Tilton Town Dam is located in the Towns of Tilton and Northfield, New Hampshire. The dam is a run-of-the-river dam spanning the Winnipesaukee River approximately 5.3 miles above its confluence with the Pemigewasset River. The centerline of the river serves as the boundary between Tilton and Northfield. The Merrimack River originates at the confluence of the Winnipesaukee and Pemigewasset Rivers in Franklin, New Hampshire. Tilton Town Dam is shown on U.S.G.S. Quadrangle, Penacook, New Hampshire with coordiantes approximately at N 43° 26' 33", W 71° 35' 43". Tilton is located in Belknap County; Northfield is located in Merrimack County. (See Location Map, page vii.)

- Description of Dam and Appurtenances. Tilton Town Dam is a low, run-of-the-river dam totaling 192 feet in length and having a hydraulic height of 13 feet. The north abutment of the dam is located in Tilton and consists of a concrete sluiceway with discharge controlled by a timber gate. The timber gate is 5.6'H x 6'W and has an invert 3.5 feet below the spillway crest. The maximum gate opening is 11.6' above the sluiceway invert. It is operated by means of a mechanical lifting device (chain hoist). A 10-inch intake is located on the upstream side of the wingwall of this gate structure. This intake provides water for use in the Arthur S. Brown Manufacturing Company plant building which is located just adjacent to the north abutment. The spillway consists of a timber frame structure about 124 feet in length. The spillway crest is 9.6 feet above the downstream toe of the dam. abutment of the dam is located in Northfield and consists of a concrete sluiceway with discharge also controlled by a timber gate. The timber gate is 8.3'H x 10'W and has an invert 6 feet below the spillway crest. The maximum gate opening is 11.7' above the sluiceway invert. This gate is now raised mechanically by an external movable power source (bucket loader) but could be raised by a heavy chain hoist. A concrete box inlet structure is constructed on the upstream side of the south sluiceway wingwall. There is a 10-inch pipe inlet from this structure supplying an auxiliary fire pump in the Surrette Storage Battery Company, a factory on the south side of the river immediately downstream of the dam.
- c. Size Classification. Small (hydraulic height 13 feet; storage 50 acre-feet) based on height and storage ( < 40 feet and ≥ 50 to < 1,000 acre-feet) as given in Recommended Guidelines for Safety Inspection of Dams.
- d. Hazard Classification. Significant Hazard. A major breach at top of dam would probably result in the possible loss of a few lives and could cause appreciable property damage. (See Section 5.1 f.)
- e. Ownership. Tilton Town Dam was constructed prior to 1886. The earliest ownership recorded appeared on a New Hampshire Water Resources Board (NHWRB) inspection report dated 8/30/34. This report states that the Tilton (north) side of the dam is owned by the Public Service Company of New Hampshire and the Northfield (south) side by the Elm Mills Woolen Company. This ownership apparently remained unchanged until the Town of Tilton purchased the dam and property March 8, 1968.
- f. Operator. The current owner and operator of the Tilton Town Dam is the Town of Tilton, Town Hall, 145 Main Street, Tilton, New Hampshire 03276. Phone: (603) 286-4425.
- g. Purpose of Dam. The purpose of the original construction of the dam is not known. Sometime prior to 1934 the dam was utilized for power generation to both of its co-owners, Public Service Company of New Hampshire and Elm Mills Woolen Company. A NHWRB inspection report of December 17, 1934 states that the

Tilton side of the dam (Public Service Company) was not operating. The pondage behind the dam is presently used to supply several plants with industrial process water. A NHWRB memo of October 15, 1976 reflects this pondage was also used to dilute sewage from a few lines which discharge into the pond above the dam. An article which appeared in the Concord Monitor, Monday, April 30, 1979, reflects that this dam is currently being considered as a future source of hydroelectric power. (See Appendix B.)

- Design and Construction History. Tilton Town Dam was constructed sometime prior to 1886. No information was disclosed regarding the original design and construction of the dam. A NHWRB inspection report dated 6/25/36 reported the center portion of the dam was damaged in the flood of March 1936 but was repaired immediately. A NHWRB sketch dated 9/18/39 reflects a 107-foot long spillway, two adjacent gate structures forming the south abutment, and one wood flume forming the north abutment. Further information was obtained from a study done by Fenton G. Keyes Associates, Hydraulic Calculations for the Winnipesaukee River from Lake Winnipesaukee to the Merrimack River, prepared for the New England Division, U.S. Army Corps of Engineers, January, 1957. The above report reflects two gates exist, one 8.6-foot wide gate on the north end and one 8.2-foot wide gate on the south end. spillway in this report was noted to be 124.5 feet in length. Therefore, structural modifications occurred between the years of 1939 and 1957. Additional structural changes in the gate sluiceways have occurred after 1957. The structure as seen on the visual inspection consisted of a sluiceway with a 5.6'H x 6'W gate on the north side which was reported to have been constructed in 1969 and a sluiceway with a 8.3'H x 10'W gate on the south side which was reported to have been constructed in 1974. These two dates were obtained orally from the owner.
- i. Normal Operating Procedures. No written operating procedures were disclosed. The gates are normally opened in the spring and closed in mid July. The water level is watched by a staff member of the Arthur S. Brown Manufacturing Company plant and conditions are relayed to the town. The Town of Tilton Road Agent operates the gates as deemed necessary.

#### 1.3 Pertinent Data

a. Drainage Area. The drainage area consists of 473 square miles (302,720 acres) of varied terrain and includes the 363 square mile Lake Winnipesaukee drainage area. The Winnipesaukee River originates at Lake Winnipesaukee and flows in a southwesterly direction through Paugus Bay, Opechee Bay, Winnisquam Lake, Silver Lake and the communities of Laconia, Belmont, Tilton, Northfield and Franklin. Three dams affect flood control on the Winnipesaukee River. The Lakeport Dam, located between Paugus Bay and Opechee Lake, regulates the elevation of Paugus Bay and Lake Winnipesaukee. The drainage area to Lakeport Dam is 363 square miles. Avery Dam, located between Opechee Lake and Lake Winnisquam, has a drainage

area of 374 square miles. Lochmere Dam, which regulates the water surface of Lake Winnisquam, carries a drainage area of 428 square miles. Tilton Town Dam is located downstream of these three flood control dams on the Winnipesaukee River.

#### b. Discharge at Damsite

- (1) Outlet works (sluices) 5.6' H x 6'W timber gate @ invert elevation of 436.9' MSL. 8.3'H x 10'W timber gate @ invert elevation 434.4' MSL. Combined capacity at top of dam 3100 cfs @ 443.4' MSL.
- (2) The maximum discharge at damsite A U.S.G.S. gaging station, having a drainage area of 471 square miles, is located 0.4 miles upstream of the dam and has a record since January 1937. The maximum recorded discharge at the gage is 3,810 cfs which occurred during the September 1938 flood. Therefore, the discharge at the dam during the 1938 flood was probably in excess of 3,810 cfs. However, a greater discharge may have occurred during the 1936 flood which is the largest of historical record on the Winnipesaukee River, evidenced by high water marks. Also recorded are discharges of 3,720 cfs and 3,700 cfs which occurred in 1953 and 1954, respectively.
- (3) Ungated spillway capacity @ top of dam elevation 2,200 cfs @ 443.4' MSL.
- (4) Ungated spillway capacity @ test flood elevation 5,605 cfs @ 446' MSL
- (5) Gated spillway capacity @ top of dam elevation not applicable
- (6) Gated spillway capacity @ test flood elevation not applicable
- (7) Total spillway capacity @ test flood elevation 5,605 cfs @ 446' MSL
- (8) Total project discharge @ test flood elevation 7,570 cfs @ 446' MSL (with gates closed); 7,570 cfs @ 444.3' MSL (with gates open)

#### c. Elevation (ft. above MSL)

- (1) Streambed at centerline of dam 430.8 (at downstream toe)
- (2) Maximum tailwater During the September 1938 flood with a discharge of 3,810 cfs maximum tailwater is estimated to have been at 433.5' MSL.

- (3) Upstream invert north abutment sluiceway 436.9 Upstream invert south abutment sluiceway 434.4
- (4) Recreation pool not applicable
- (5) Full flood control pool not applicable
- (6) Spillway crest 440.4
- (7) Design surcharge (Original Design) unknown
- (8) Top of dam 443.4
- (9) Test flood pool 446

#### d. Reservoir (feet)

- (1) Length of maximum pool 2700
- (2) Length of spillway crest pool 1450
- (3) Length of flood control pool not applicable

#### e. Storage (acre-feet)

- (1) Recreation pool not applicable
- (2) Flood control pool not applicable
- (3) Spillway crest pool 16 (approximate)
- (4) Top of dam 50 (approximate)
- (5) Test flood pool 80 (approximate)

#### f. Reservoir Surface (acres)

- (1) Recreation pool not applicable
- (2) Flood control pool not applicable
- (3) Spillway crest 4 (approximate)
- (4) Test flood pool 13 (approximate)
- (5) Top of dam 9 (approximate)

#### g. Dam

- (1) Type timber frame spillway and wooden upstream deck with concrete sluiceways and abutments at either end.
  - (2) Length 192'

- (3) Height 13' (structural height)
- (4) Topwidth varied
- (5) Side slopes U/S spillway 4H:1V U/S abutments vertical; D/S spillway and abutments vertical.
  - (6) Zoning none
  - (7) Impervious core none
  - (8) Cutoff none
  - (9) Grout curtain none
- h. <u>Diversion and Regulating Tunnel</u> not applicable (See j. below.)

#### i. Spillway

- (1) Type timber frame with wooden decking
- (2) Length of weir 124'
- (3) Crest elevation 440.4' MSL
- (4) Gates none
- (5) U/S Channel the upstream channel consists of the Winnipesaukee River. A Boston & Maine Railroad bridge and the Route #38 bridge are located 580 feet and 660 feet upstream of the dam, respectively. The Arthur S. Brown Mfg. Co. plant is located on the north side of the approach channel.
- (6) D/S Channel the downstream channel immediately below the dam is wide and has a boulder strewn and rugged exposed bedrock bottom. No trees, brush or man-made structures obstruct the immediate channel below the dam. The J.P. Stevens Company plant is located downstream on the south side of the channel. Two dams and six bridges have been constructed in the downstream reach of the Winnipesaukee River between Tilton Town Dam and its confluence with the Pemigewasset River in Franklin, New Hampshire, a distance of 5.3 miles.
- j. Regulating Outlets. The north abutment of the dam consists of a concrete gate structure with a 5.6'H x 6'W timber gate and head frame. This gate is mechanically operated by a chain hoist and it has a maximum opening of 11.6' above the sluiceway invert. On the upstream side of this structure is a 10-inch process water intake which supplies process water to the Arthur S. Brown Mfg. plant.

The south abutment also consists of a concrete gate structure with an 8.3'H x 10'W timber gate and head frame. This gate is now raised mechanically by a movable power source (bucket loader). It has a maximum opening of 11.7 feet above the sluiceway invert. On the upstream face of the structure is a 10" intake pipe. This intake supplies water to an auxiliary fire pump for the Surrette Storage Battery Co., a factory on the south bank of the river immediately downstream of the dam.

#### SECTION 2 ENGINEERING DATA

#### 2.1 Design

No original design data were obtained for Tilton Town Dam.

#### 2.2 Construction Records

No written construction records were disclosed. The owner stated that the north sluiceway was constructed in 1969 and the south sluiceway was constructed in 1974.

#### 2.3 Operation

No engineering operational data were obtained.

#### 2.4 Evaluation

- a. Availability. A search of the files of the NHWRB and direct contact with the owner revealed only a limited amount of recorded information.
- b. Adequacy. Because of the limited amount of detailed data available, the final assessments and recommendations of this investigation are based on the hydrologic and hydraulic calculations and the visual inspection.
- c. <u>Validity</u>. No original engineering data were obtained. Hydrologic and hydraulic studies done on the Winnipesaukee River (discussed in Section 5.1 b.) were reviewed and validated.

### SECTION 3 VISUAL INSPECTION

#### 3.1 Findings

- a. General. Tilton Town Dam is a low, run-of-the-river dam which impounds a reservoir fo small size on the Winnipesaukee River located immediately downstream of Tilton Village. The watershed above the reservoir is rolling and partially wooded. The Winnipesaukee River is the outlet for Lake Winnipesaukee and flows through Winnisquam Lake and Silver Lake a few miles upstream of Tilton Town Dam. The dam is 5.3 miles upstream from the confluence of the Winnipesaukee and Pemigewasset Rivers. There are two dams and six bridges downstream of Tilton Town Dam on the Winnipesaukee River.
- b. <u>Dam</u>. Tilton Town Dam consists of a timber-frame spillway with a wooden upstream decking placed between two concrete gated outlet structures. (See Appendix C Figures 2 and 3). The dam totals 192 feet in length and has a hydraulic height of 13 feet.

During the initial inspection performed April 6, 1979, about one foot of water was flowing over the crest of the spillway. It was noted that the level of the water flowing over the crest was not uniform along the length of the spillway. (See Appendix C - Figure 4.) From this observation it was inferred that local failures have occurred along the length of the structure. A subsequent inspection was performed April 24, 1979, in conjunction with representatives of the New Hampshire Water Resources Board (NHWRB) and the owner, the Town of Tilton. The NHWRB restricted discharge at Lakeport Dam, reducing flow in the Winnipesaukee River. The Town of Tilton opened both gates at Tilton Town Dam and the impoundment behind the dam was lowered so that little water was discharging over the crest of the dam. This enabled a more thorough inspection on the structural condition of the dam.

The timber frame of the dam is, in part, in poor condition and badly deteriorated. Confirmation of some local support failures was made. (See Appendix C - Figure 5.) The timber decking over the upstream face is in very poor condition. (See Appendix C - Figure 6.) Major quantities of water are pouring through large holes in the decking and lesser quantities through leaks along the entire length of the spillway. (See Appendix C - Figure 5.) Near the south abutment is a hole several feet in diameter in the decking and a whirlpool several feet in diameter was observed over this hole. (See Appendix C - Figure 7.)

Extensive outcrops of bedrock were observed on the south bank of the reservoir immediately upstream of the dam. No outcrops of rock were observed on the north bank. In the available records, one cross section shows the timber structure as being founded on "ledge", and one written record indicates that the foundation of the dam is "hardpan". On the basis of a visual inspection from the shoreline downstream of the dam it appears that most of the timber framing of the dam rests on bedrock and that parts of it rests on large boulders.

No evidence of seepage or other problems were observed at the south abutment. A mill building is located at the north abutment. An inspection of the basement of that building did not reveal any signs of seepage or other problems.

c. Appurtenant Structures. Two concrete sluiceways pass through the dam, one at each abutment. (See Appendix C - Figures 8, 9 and 10.) The north sluiceway was reported to have been constructed in 1969; the south sluiceway in 1974.

The sluiceway on the north side is 6 feet wide and the invert of the channel is 3.5 feet below the dam crest. The sluiceway on the south side is 10 feet wide and 3.3 feet below the dam crest at the inlet and 6 feet below crest at the timber gate. Each sluiceway has steel gate slots, cast into the side approximately 12 feet from the upstream end.

Both timber gates were raised at the time of inspection and were observed to be in a deteriorated condition. Several planks near the bottom of the south sluiceway gate were damaged by a beaver. The planking at the bottom of the north sluiceway gate is in deteriorated condition. Several of the planks are bowed downstream and one of the planks is broken. The steel gate slots were observed to have surface rust but were otherwise in good condition.

The concrete walls of the sluiceways were observed to be in good condition except for some minor erosion at the inlet end.

A concrete box inlet structure is constructed on the upstream side of the south sluiceway wingwall. There is a 10-inch pipe inlet from this structure supplying an auxiliary fire pump in a building immediately downstream of the dam. (See Appendix C - Figure 9.) The concrete box appears to have been constructed at the same time as the sluiceway.

A sand fill has been placed between the north abutment and the sluiceway structure at the north end of the dam. A sinkhole, about 18 inches in diameter, was observed at the downstream side of this fill. (See Appendix C - Figure 11.) According to the maintenance manager of the Arthur S. Brown Mfg. Co., the sand fill was placed on top of a wood decking which, in turn, is above the water intake pipe to the mill. Collapse of the decking may have caused the sinkhole.

- d. Reservoir Area. The watershed above the reservoir is rolling and partially wooded. Numerous structures are built close to the edge of the reservoir immediately upstream of the dam in the Town of Tilton. The Winnipesaukee River, spanned by the Tilton Town Dam is the outlet for Lake Winnipesaukee and flows through Winnisquam Lake and Silver Lake a few miles upstream from Tilton Town Dam. The Arthur S. Brown Manufacturing Company building is located immediately upstream of the dam on the north approach channel. (See Appendix C Figure 12.)
- e. <u>Downstream Channel</u>. The downstream channel is broad, unobstructed by trees and brush, with a boulder and rugged exposed bedrock bottom. (See Appendix C Figure 13.) A mill building is located on the north side of the channel immediately downstream of the dam. There are also two dams and six bridges spanning the river between Tilton Town Dam and its confluence with the Pemigewasset River about 5.3 miles downstream.

#### 3.2 Evaluation

Based on the visual inspection, Tilton Town Dam appears to be in poor condition. The timber frame dam is so badly deteriorated that it may collapse, particularly during seasonal high water flows. If there are pollutants in the silt behind the dam they could cause significant environmental problems downstream if the dam failed or was breached. The deteriorated condition of the wooden gates could cause loss of water in the reservoir which would also cause the loss of process water to the Arthur S. Brown Manufacturing Company and loss of fire protection water to the manufacturing facility on the south side of the dam.

# SECTION 4 OPERATIONAL PROCEDURES

#### 4.1 Procedures

No written operational procedures were obtained for Tilton Town Dam. Flow conditions are watched by a staff member of the Arthur S. Brown Mfg. Company and relayed to the town. The gates are normally open in the spring and closed in mid July. A hand winch is used to open the north gate. Because of the friction caused by hydrostatic pressure the gate on the south side must be raised by an external source of power (bucket loader or crane).

#### 4.2 Maintenance of Dam

The Town of Tilton is responsible for the maintenance of the dam.

#### 4.3 Maintenance of Operating Facilities

No formal maintenance is performed.

#### 4.4 Description of Any Warning System in Effect

No written warning system was revealed.

#### 4.5 Evaluation

The present operational and maintenance procedures are not adequate to ensure proper operation of the gates during high flows. The maintenance procedures are inadequate to ensure that all problems encountered can be remedied within a reasonable period of time. The owner should establish a written operation and maintenance procedure as well as establishing a warning system to follow in event of emergency conditions.

### SECTION 5 HYDROLOGIC/HYDRAULIC

#### 5.1 Evaluation of Features

- a. General. Tilton Town Dam is a low, run-of-the-river dam which impounds a reservoir of small size. The dam has a timber frame spillway with a wooden upstream decking placed between concrete abutments. If this structure were breached, the failed portion of the timber structure could become lodged in a number of other dams and bridges downstream of the dam. Two dams and six bridges are located in the reach on the Winnipesaukee River between Tilton Town Dam and its confluence with the Pemigewasset River, a distance of 5.3 miles.
- b. Design Data. The available data pertinent to the Tilton Town Dam comes from five primary sources:
- (1) The New Hampshire Water Resources Board (NHWRB) files on the dam;
- (2) Hydraulic Calculations for the Winnipesaukee River from Lake Winnipesaukee to the Merrimack River, prepared by Fenton G. Keyes Associates for the Corps of Engineers, New England Division, in 1957;
- (3 and 4) The back up files for the Flood Insurance Studies of Tilton and Northfield, New Hampshire, prepared for the Federal Insurance Administration by Hamilton Engineering Associates, Inc. of Nashua, N.H. and Anderson-Nichols & Company, Inc. (ANCo.) of Concord, New Hampshire, respectively.
- (5) Hydraulic Engineering Analysis for Evaluating Flood Stage Reduction on the Winnipesaukee River, New Hampshire, prepared by ANCo for the Corps of Engineers, New England Division, December 1978.

The following is a summation of data pertinent to Tilton Town Dam found in each of the above references:

- Reference 1: It is the opinion of the NHWRB that this structure has been in poor condition since 1934; NHWRB feels that this dam could fail at any time and should be removed. (See Appendix B and Section 6.1 c.)
- Reference 2: The final recommendations of this study with reference to Tilton Town Dam was that the spillway section of the dam be lowered 3.50' and this section be replaced with flashboards or crest gates which can easily be removed or dropped during high flows.

References 3 and 4: HEC-2 step-backwater computations reflect that with a discharge of 7,570 cfs an elevation of 445.4' msl would be reached. The hydraulic input of this study was reviewed and evaluated. In order to reflect existing conditions, backup from the 1978 ANCo study (Reference 5) was utilized in developing a stage-discharge relationship for Tilton Town Dam. The rating curve developed in the FIS studies, References 3 and 4, ranges 0.6 to 1.0 foot lower than the rating curve developed for this inspection study.

Reference 5: The recommendation of this study was to replace the dam with a weir having a crest elevation at least 1.5 feet lower than the existing spillway. From trial HEC-2 runs through this area, it was determined that lowering the spillway would reduce flooding upstream of the dam to the Route 38 bridge crossing. Several companies utilize the pondage for process water. Breaching the dam would create a hardship, therefore, this was not recommended as an alternative.

- Experience Data. In a NHWRB report of 6/25/36 it was reported that the center portion of the dam was damaged in the flood of March 1936 but was repaired immediately. No recorded discharge was disclosed for this flooding event. The U.S.G.S. gage in Tilton, N.H. came into operation January 1937 and remains in current use. The maximum recorded discharge occurred during the September 1938 flood and was recorded to be 3,810 cfs. No records were found that reported any sustained damages to the dam. The 1936 flood, however, is reported to be the largest of historical record on the Winnipesaukee River as demonstrated by high water marks in the area of the dam. The 1938 profile along with the 1936 high water marks are shown on the Winnipesaukee River Plan and Profile, Sheet 1, U.S. Engineer Office, February 1939. Recorded discharges of 3,720 cfs and 3,700 cfs which occurred in 1953 and 1954, respectively, were also obtained.
- d. <u>Visual Observations</u>. The dam is in poor condition. The timber spillway has an uneven crest caused by some local support failures. The timber decking on the upstream face is in very poor condition.
- e. Test Flood Analysis. Tilton Town Dam is classified as being small in size having a hydraulic height of 13 feet and a maximum storage capacity of 50 acre-feet; the dam was determined to have a Significant Hazard Classification. Using the Recommended Guidelines for Safety Inspection of Dams, the test flood was determined to be ½ Probable Maximum Flood (PMF).

The test flood inflow cannot simply be determined by use of the PMF guide curves due to the complexity of the hydrologic and hydraulic conditions which comprise the Winnipesaukee River drainage basin. Flooding on the Winnipesaukee River and its associated bays and lakes is to a large extent controlled by Lochmere Dam on Lake Winnisquam, Avery Dam on the Winnipesaukee River, and Lakeport Dam between Opechee Lake and Paugus Bay. Referring to the Northfield Flood Insurance Study, peak discharges

on the Winnipesaukee River were determined at Lakeport Dam, Avery Dam, and Lochmere Dam using various hydrologic methods. The peak discharge at the Tilton gage during a 500-year storm was determined to be 7,570 cfs. This gage is located 0.4 miles upstream of Tilton Town Dam. This 500-year flood flow, being approximately the test flood, was utilized in determining the overtopping potential of Tilton Town Dam. The discharge at Lakeport Dam during a flooding event of this magnitude was determined to be 4,300 cfs.

The overtopping analyses indicates that the dam would be overtopped by 2.6 feet (5.6 feet over spillway crest) during the test flood, assuming both gates closed. The maximum spillway capacity at top of dam is 2200 cfs which is 29 percent of the test flood discharge, assuming the dam stayed intact during a flooding event of this magnitude. Assuming both gates closed, a test flood discharge of 7,570 cfs would overtop the dam by 0.9 foot (3.9 feet over spillway crest).

Dam Failure Analysis. A major breach at top of dam would result in a discharge of about 4,315 cfs. This flow is similar to the 4,475 cfs used in Reference 5 (see 5.1 b.). Therefore, the profile developed with this discharge could be utilized to estimate the level of probable damages caused by dam failure at top of dam. Included on this profile are elevations of key damage points. From the profile, the only damage caused by a breach would be the Arthur S. Brown Mfg. Company building which is located adjacent to the north abutment of the dam. A portion of this plant is located in the channel and is the working area for two people. The maintenance garage located at the south abutment at times is occupied. Therefore, loss of life is possible. Property damage could be appreciable. Plants which utilize the pondage for process water would be without water. Loss of water could cause substantial damage to the boilers in the Arthur S. Brown Mfg. Company plant. The pondage also supplies water to an auxiliary fire pump. Therefore, Tilton Town Dam was classified Significant Hazard.

### SECTION 6 STRUCTURAL STABILITY

#### 6.1 Evaluation of Structural Stability

a. <u>Visual Observations</u>. The timber decking on the upstream face of the dam is in very poor condition. Large quantities of water are leaking through holes in the decking at many locations. Near the south abutment there is a hole several feet in diameter in the decking and there is a whirlpool several feet in diameter over the hole. The timber frame of the dam is also in poor condition.

Several planks near the bottom of the south sluiceway gate were damaged by a beaver. The planking near the bottom of the north sluiceway gate is in poor condition. Several of the planks are bowed downstream and one of the planks is broken. A small sinkhole in the fill above the intake pipe from the reservoir to the industrial plant was observed.

b. Design and Construction Data. A sketch dated 9/18/39 shows a cross section of the dam with vertical 8" x 10" posts on the downstream side, an upstream decking consisting of a double thickness of 2-inch planks and inclined at about 4H:1V, and 8" x 10" horizontal ties in both the longitudinal and transverse directions. This sketch indicates that the timber frame is founded on "ledge". It appears that this sketch shows the condition of the dam as it existed at the time of an inspection of the same date. However, there is an apparent discrepancy because the written inspection report indicates that the dam is founded on hardpan.

It was reported that the north sluiceway was constructed in 1969 and the south sluiceway in 1974.

c. Operating Records. Several references in the available records reflect that the timber-structure of the dam has been in poor condition at various times in the past:

Dec. 17, 1934 dam inspection report - Dam, timber A frame, condition poor, should be repaired.

June 25, 1936 dam inspection report - Condition poor. Center portion of dam damaged in flood of Mar. 1936. Repaired immediately.

Sept. 18, 1939 dam inspection report - Condition poor.

June 18, 1951 dam inspection report - Timber frame...is rotting and will go in a few years.

May 5, 1976 note to WRB file - Whirlpool upstream of the timber dam. It appeared that perhaps a section of planking had broken and was letting water through the underside of the dam.

Oct. 15, 1976 WRB internal memo Evidence of...large whirlpool.... The main dam is
in poor condition; the entire decking as well as
all the support framing should be replaced. Water
was going through the decking in several locations
and the crest of the dam sags at the location
where the repairs were made some time ago indicating that their (sic) has been a structural
failure of the support timbers.... It is my
opinion that the structure could fail at any time.

- d. Post-Construction Changes. See 1.2 h.
- e. Seismic Stability. The dam is located in Seismic Zone 2 and in accordance with the recommended Phase I guidelines does not warrant seismic analysis.

## SECTION 7 ASSESSMENT, RECOMMENDATIONS, AND REMEDIAL MEASURES

#### 7.1 Dam Assessment

- a. Condition. The visual inspection and the comments made in previous dam inspection reports, memos, and letters which are available in the N.H. Water Resources Board files indicate that Tilton Town Dam is in poor condition. The principal concern with respect to the integrity of the dam is the poor condition of the timber frame structure and wooden decking of the overflow section of the dam. The gate on the south side must be raised by an external mechanical force such as a bucket loader, crane, etc. and therefore, cannot be removed quickly and easily.
- b. Adequacy of Information. The information available is adequate to assess the condition of the dam. The conclusions about the condition of the dam are based primarily on the results of the visual inspection.
- c. <u>Urgency</u>. The recommendation, remedial measures, or alternative in 7.2, 7.3, or 7.4 respectively, should be implemented by the owner within one year after receipt of this Phase I report.
- d. Need for Additional Information. No additional information is needed to assess the condition of this dam.

#### 7.2 Recommendation

The owner should engage a registered professional engineer to design and supervise appropriate repairs to the sinkhole over supply pipe in south abutment and to the existing structure such as frame, decks, gates, lift mechanisms, etc. Attention should should be paid to the possibility that the silt collected behind the dam may contain pollutants which could affect the downstream reaches of the river.

#### 7.3 Remedial Measures

a. Operating and Maintenance Procedures. Establish a surveillance program for use during and immediately following periods of heavy rainfall, and also a warning program to follow in case of emergency conditions. Institute a program of annual technical inspection after dam is repaired or replaced.

#### 7.4 Alternative

The owner should engage a registered professional engineer to design and supervise construction of a suitable replacement dam.

#### APPENDIX A

VISUAL INSPECTION CHECKLIST

# VISUAL INSPECTION CHECKLIST PARTY ORGANIZATION

PR	ROJECT Tilton Town Dam, N.H.		DATE April	<u>6,</u> 1979	•
			TIME	<u>A.</u> M.	-
			WEATHER Cold,	cloudy	· ·
			W.S. ELEV.	Ú.S.	DN.S.
PA	RTY:			441	433.9
1	Warren Guinan (4/24/79)	_ 6			
2.	Stephen Gilman (4/24/79)				
3.	Leslie Williams	_ 8			
4.	Ronald Hirschfeld (4/24/79)	_ 9			
5.	Pattu Kesavan	_ 10			
	PROJECT FEATURE		INSPECTED BY	REM	ARKS
1	Hydrology/Hydraulics		W. Guinan/L. W	illiams	
2	Structural Stability		S. Gilman		
3	Soils & Geology		R. Hirschfeld		
8					
- V + -			······································		

#### PERIODIC INSPECTION CHECKLIST DATE April 6 & 24, 1979 Tilton Town Dam, N.H. PROJECT \_\_ PROJECT FEATURE Intake Structure & Channel NAME -DISCIPLINE \_\_ NAME \_\_\_\_\_ AREA EVALUATED CONDITION OUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE a. Approach Channel Slope Conditions Good Bottom Conditions Not visible beneath water surface Rock Slides or Falls None Log Boom None Debris Some observed on upstream face below water surface Condition of Concrete Lining Good Drains or Weep Holes None apparent Intake Structure Condition of Concrete Good, only surface laitance eroded away SCHOOL STORE STORE STORE STORE Fair, embedded steel surface rusted no paint 3" wood weathered - several planks Stoplogs deteriorated and bowed. One plank on north side broken

#### PERIODIC INSPECTION CHECKLIST

PROJECT Tilton Town Dam	DATE April 6 & 24, 1979		
PROJECT FEATURE Outlet Structure & Ch	annel NAME		
DISCIPLINE			
AREA EVALUATED	CONDITION		
OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL	Concrete Sluiceways		
General Condition of Concrete	Good		
Rust or Staining	Only at embedded steel items		
Spalling	None		
Erosion or Cavitation	Only surface laitance eroded where in contact with water		
Visible Reinforcing	None		
Any Seepage or Efflorescence	None observed		
Condition at Joints	Good		
Drain holes	None apparent		
Channel			
Loose Rock or Trees Overhanging Channel	Small trees overhanging north side, but channel is wide and unobstructed.		
Condition of Discharge Channel	Good		

#### PERIODIC INSPECTION CHECKLIST DATE April 6 & 24, 1979 Tilton Town Dam, N.H. PROJECT PROJECT FEATURE Spillway Weir NAME \_\_\_\_\_ DISCIPLINE \_\_\_ NAME \_\_\_\_ AREA EVALUATED CONDITION OUTLET WORKS - SPILLWAY WEIR, APPROACH Timber frame spillway with decking on AND DISCHARGE CHANNELS upstream face a. Approach Channel General Condition. Good Loose Rock Overhanging Channel None Trees Overhanging Channel None Floor of Approach Channel Not visible beneath water surface Wood frame-badly deteriorated, one large b. Weir and Training Walls hole in deck, many small ones. Crest of dam is irregular and sagged. General Condition of Concrete Rust or Staining Only at tie holes and embedded steel supports Spalling None visible Any Visible Reinforcing None Any Seepage or Efflorescence None Drain Holes Nane c. Discharge Channel General Condition Good Loose Rock Overhanging Channel None Trees Overhanging Channel Small trees overhanging north side, but channel is wide Floor of Channel Not visible beneath water surface Other Obstructions None

#### PERIODIC INSPECTION CHECKLIST

PROJECT Tilton Town Dam, N.H.	DATE April 6 & 24, 1979
PROJECT FEATURE Service Bridge	NAME
DISCIPLINE	NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - SERVICE BRIDGE	
a. Super Structure	
Bearings	Not applicable
Anchor Bolts	Not applicable
Bridge Seat	Not applicable
Longitudinal Members	Not applicable
Underside of Deck	Not applicable
Secondary Bracing	Not applicable
Deck	Wood plank 3" thick untreated in weathered condition
Drainage System	Not applicable
Railings	Not applicable
Expansion Joints	Not applicable
Paint	Not applicable
b. Abutment & Piers	See Outlet Structure - Outlet Works
General Condition of Concrete	
Alignment of Abutment	
Approach to Bridge	
Condition of Seat & Backwall	

# PROJECT Tilton Town Dam, N.H. PROJECT FEATURE Reservoir

DATE April 6, 1979

NAME L. Williams

AREA EVALUATED	REMARKS
Stability of Shoreline	Good
Sedimentation	None observed
Changes in Watershed Runoff Potential	None significant
Upstream Hazards	Route 38 and Boston & Maine Railroad
Downstream Hazards	Bridge Part of Arthur S. Brown Mfg. Co. plant
Alert Facilities	None posted
Hydrometeorological Gages	U.S.G.S. gage 0.4 miles upstream of dam in Tilton
Operational & Maintenance Regulations	None posted

APPENDIX B

ENGINEERING DATA

# Tilton Dam May Create Energy Again

By ANDREW MEANS Monitor Staff Writer

TILTON - The selectmen will soon have to decide the future of the townowned dam across the Winnipesaukee River, and one option may be to restore it as a source of hydroelectricity.

Three of the operators of the Sulloway Mill hydroelectric plant in Franklin have approached the Tilton selectmen to find out if the town is interested in selling them. the dam to generate electricity.

The three, Ted Larter of Dunstable, Mass., Tony Turgeon of Tilton and John Clement of Franklin, have been asked by selectmen to provide more details of any restoration plan they may have for the i.dam.

The N.H. Water Resources Board has been concerned about the condition of the dam for some time. The U.S. Army Corps of Engineers is inspecting the dam this

week, said Water Resources Engineer Vernon Knowlton.

"After we receive their report we will order the town to either repair it or remove it," said Knowlton, adding that "any interest from anyone wanting to generate hydroelectricity would be helpful."

The dam is over 100 years old, he said, but has not been used to generate power since the ownership was transferred from Public Service Co. of N.H. to the town of happy to buy the water rights alone. Tilton over 10 years ago.

It is currently used to provide water. mainly for processing and emergency firefighting purposes, in two factories, Ar. . Besides the Franklin plant, Larter also

, could generate 800 to 1,000 kilowatts an power from this source, and Public Serhour. This could be sold to a power compa- vice buys from the Sulloway Mill plant. ny such as Public Service or, conceivably, 7.3 The viability of such plants depends used as an emergency standby system for largely on energy policy in general, he the town.

But Larter stressed that the feasibility is still being studied.

"As a matter of a fact it may not be big enough to make it worthwhile," he said.
"All we want to know is whether the town would be willing to convey it over to us for us to start work on it." enough to make it worthwhile," he said. . us to start work on it."

Larter said restoration would be a private operation. In the unlikely event that the selectmen decided to keep the dam. and repair it, he said, he would be just as

"We have just so much money to spend," he said. "There's a lot of liability that goes along with (the dam)."

thur S. Brown Co. and Surrette Batteries. Jowns an operation at Goodrich-Falls in If the dam is restored, said Larter, it Bartlett, N.H. Electric Cooperative buys

said.

# State of New Hampshire Water resources board

CONCORD 03301

October 15, 1976

Re. Dam #237.02

Board of Selectmen Town Office Tilton, N. H. 03276

#### Gentlemen:

In answer to telephone requests from the town of Tilton for assistance in lowering the water in the Winnipesaukee River so that the Town could make repairs to their dam located adjacent to the Brown Manufacturing Company, an engineer of this office did a preliminary inspection of that dam on October 14th with Mr. Manning, the Road Agent of the Town.

Mr. Manning explained that for sometime the Town has been trying to repair a hole in the decking and with the gates on the dam have not been able to control the flow of water which is at present at 280 cfs. We explained to Mr. Manning and to members of the Board of Selectmen that with our dam at Lochmere being under reconstruction, it is impossible for us to restrict the flow of water from Lake Winnisquam.

During the inspection our engineer, Mr. D. Rapoza, discovered that in addition to the deck which is in extremely poor condition, many of the upright braces supporting the A-frames and the connecting timbers between the A-frames are in a state of decay. One section of the dam's crest is already sagging indicating a structural failure in that section of the timber dam. In our review of the inspection report and our file on this dam, it is our opinion that a loss of the supporting timbers could cause a failure of a portion of this dam "at any time". The uncertainty of when such a failure could occur creates a problem at which the Town is left with a decision to be made, we feel, in the rear future.

This Dam #237.02 in the files of the Water Resources Board is classified as a menace structure. This classification indicates that due to its height, storage, and location, failure of the dam could jeopardize the lives and safety of the public. This office has reviewed this classification and feel that a major liability connected with this dam would be following the failure portion of the timber structure could become lodged in a number of bridges and other dams downstream of Tilton perhaps causing structural damage to these facilities.

In reviewing our files and discussing this matter with members of the Board of Selectmen, the Water Resources Board has been on record in the past indicating that if this dam is not serving a useful purpose it would benefit the public if it was removed or lowered substantially since the present dam maintains a high water level throughout the town which reduces the ability to pass flood waters down the Winnipesaukee River. It is our present understanding that the existing dam helps to improve a sewer condition in the town and provides a reservoir for process water for local industry. During the immediate future if the town reconsiders reconstructing this dam, perhaps they should consider reconstructing the dam at a lower height or supplying the water needs to their industry from a different source.

This office wishes to cooperate with the town of Tilton in any way in this matter; and our staff will be available to meet with the town's engineers to discuss this problem at your convenience.

Due to the nature of the condition of this dam, the New Hampshire Water Resources Board requests the town of Tilton to notify us within the next few weeks of its plans to take corrective action regarding the situation that presently exists. Except for what nature might create, the flows in the Winnipesaukee River will not be increased by the operations of Lake Winnipesaukee until the middle of December at which time the flow will be increased to approximately 1,000 cfs which would make repairs to this dam extremely expensive.

Sincerely,

George M. McGee, Sr. Chairman

GMM/VAK:L

DATE: October 15, 1976

TO: Vernon A. Knowlton, Chief Water Resources Engineer

FROM: Donald M. Rapoza, Civil Engineer

SUBJECT: Dam repairs on Town owned structure (Dam #237.02)

On October 14, 1976 I met with Mr. Raymond Manning, Road Agent for the Town of Tilton, regarding the repairs he wish to make on the town owned dam (#237.02) on the Winnipesaukee River in Tilton.

Some time ago, the town repaired a hole in the wooden deck planking with a weighted (manhole cover or frame) sheet of plywood. With the passage of time, the plywood repairs have not solved the problem, as evidence of the large whirlpool at the location of the plywood.

Mr. Manning has opened both gates at the structure in order to lower the pool elevation and with the present flow the pool has only dropped approximately 2 to 3 feet below the crest of the spillway. Mr. Manning wanted to know if we could reduced the flow in the Winnipesaukee River so that the town can make the repairs in relatively shallow water.

I also spoke with Mr. Frank Ponton, Maintenance Supervisor for the Arthur S. Brown Manufacturing Company, and he informed me that the company is not pleased with our attitude, relative to maintaining a dam at the site. He mentioned that the company is dependent on a pondage and is quite concerned in having the dam properly maintained. They have expended \$5,000.00 within the last six months to buy a pump for use of process water at the site. He stated that the town is responsible and has an obligation to keep and maintain the dam for water uses as well as diluting a few sewage lines which discharge into the pondage.

As for the dam the abutments and gates are in good condition, but the main dam is in poor condition; the entire decking as well as all the support framing should be replaced. Water was going through the decking in several locations and the crest of the dam sags at the location where the repairs were made some time ago indicating that their has been a structural failure of the support timbers.

The Town should be made aware of the present condition of the structure as it is my opinion that the structure could fail at any time.

DMR/kn

Mr. Mundy, Selectman, Town of Tilton, called regarding the development of a whirlpool upstream of the timber dam owned by the Town downstream of the bridge in the village.

They inquired whether a permit was required to lower the water to make necessary repairs.

After discussing the issue with Mr. Mundy it was my recommendation that they lower the water as soon as possible to make sure no damage was being done to the foundation. It appeared that perhaps a section of planking had broken and was letting water through the underside of the dam.

He will contact us if he feels they need assistance.

V.A.Knowlton:L

## THE STATE OF NEW HAMPSHIRE

County of Billians ss. June 34 1969
STATEMENT OF INTENT TO CONSTRUCT OR RECEIVED
RECONSTRUCT A DAM AT JUN 25 1969
TO THE WATER RESOURCES BOARD: WATER RESOURCES BOAK
In compliance with the provisions of RSA 482:3.
We, The selection of the Journ of Tillen 71 1/2.  (Here state name of person) or persons, partnership, association, corporation,
(Here state name of person/or persons, partnership, association, corporation,
etc.)
hereby state our intent to the Water Resources Board to construct, to reconstruct, to make repairs to, a dam along, or (cross out portion not applicable) across:  (Here state name of stream or body of water)
At a point Adjuctor to Tellon January (Any)  (Here give location, by distance from mouth of stream, county or
municipal boundary) in the town (s) of Ithm M.W.
in accordance with PRELIMINARY PLANS, and SPECIFICATIONS FILED WITH THIS STATEMENT AND MADE A PART HEREOF.  We, understand that more detailed plans and specifications may be requested
by the Board in conformance with RSA 482:4 and that, if such plans are requested, construction will not commence until such plans have been filed with and approved by the Board.

The purpose of the proposed construction is longer the class 5
yhich stored/water/is to be put)  (Here briefly state use to water a comment walk which stored/water/is to be put)
which stored/water/is to be put)
The construction will consist of
(Here give brief description of
work contemplated including height of dam)  Construction (details forwarded to
Witer Rossurces Brand)
All land to be flowed is not owned by applicant.
Daywood U. Athlins
Brind of Solectioners
Address Salval Sani
Tillin M. W.
<b>/</b>

Note: This statement together with plans, specifications and information and data filed in connection herewith will remain on file in the office of the Water Resources Board. This statement is to be filed in duplicate.

# RECEIVED

COCT 21 1968

120 Carrier VIN TR 20025 BOARD

## Town of Tilton

New Hampshire 03276

OFFICE OF SELECTMEN

RECEIVED

OCT 21 1968

LEV

WATER! COACO LL.

October 21,1968

Water Resources Board State Mouse Annex Concord N. H.

#### Dear Sir:

We are requesting permission to open the gate on the Berthfield side of the former Public Service Dam which the Town of Tilton now owns. We wish to lover the river for one day (next Saturday ) so hir. Dick Persons of "Person's Concrete" can estimate the cost of erecting a ten foot concrete well. It is our intention to have this wall built to stop the flow through the gate on the Tilton side and to remove this gate after the wall is completed. At this time we only wish to try to lover the the river with this one gate. It is possible later on other ways of lowering the river may have to be undertaken when the fortings and wall are to be poured. We are hoping that the reising of the northfield gate will serve our purpose at these partcular times. The gate will be closed on Sunday to allow the river to return to it's natural flow by Londay. Your urgent answer is requested. Thank you.

> Donald B. Joscelyn Chairman Board of Selectmen Tiltan h. h.

September 25, 1968

Board of Selectmen Tilton New Hamoshire 03276

Gentlemen:

Some time ago, personnel from this Board talked with you concerning repairs to the gate section in the former Elm Mills dam across Winnipesaukee River after the Town of Tilton acquired it from Public Service Company of New Hampshire. This Board expected you would notify this Board of the nature of the repairs to be undertaken. As yet, no plans have been received and the Town of Tilton now has title to this dam.

This work should be completed before the fall rains raise the river and before the spring freshets arrive. The present condition of the gate section is such that heavy flows could cause a serious failure, flooding downstream establishments for which you could be liable.

I await your plans to repair this gate section in a manner to prevent failure and insure the safety of the structure. In case you should desire it, you could arrange for Water Resources Engineer, Vernon A. Knowlton, to discuss this matter with you at Concord.

Very truly yours,

George M. McGee, Sr. Chairman

GMM/FCM/m

September 25, 1967

Mr. Eliot Priest, Vice President Public Service Company of New Hampshire Hanchester, New Hampshire

Dear Mr. Priest:

Regarding your letter of August 31, 1967 relating to the dam in the towns of Tilton and Northfield directly behind the Tilton Leather Company on the Winnipassukee River, we understand that the Town of Tilton wishes to take title to the dam and related property.

We have discussed the town retaining this dea with both Mr. Prescott, Chairman of the Board of Selectmen and Mr. Vedleigh, Chairman of the Planning Commission. We were informed that the town's interest at present is to eliminate possible health and unsanitary conditions that would be caused if this dam was removed at this time. We also understand that vater users adjacent to the dam would have to provide a more custly may of utilizing water from the stream should this dam be removed.

The Water Resources Board is of the epinion that this dam should be removed in the future if these other problems are climinated. Its removal would provide for greater discharge capacity through this section of the Winnipessukes River which, in the pest during high flood flows caused demage to proverty owners along the river upstream of the dam.

Very truly yours,

George H. McGeo, Sr. Chairman

gucg:c cc: Mrs. Prescott Mr. Wadleigh Mr. Eliot Priest, Vice President Public Service Company of New Hampshire Manchester, New Hampshire

Dear Mr. Priest:

In reply to your letter of July 7, 1967, this Board has no objection to removal of your dam directly behind Tilton Leather Company. In fact, this removal will substantially lower the flood crests through the compact open of Tilton.

Very truly yours,

George M. McGee, Sr. Chairman

gmcg:c cc: U.S.G.S...



July 7, 1967

JUL 101967

NEW HAMPSHIRE WATER RESOURCES BOARD

Mr. George M. McGee, Chairman N. H. Water Resources Board State House Annex Concord, New Hampshire

Dear Mr. McGee:

We own a dam in the Town of Tilton on the Winnipesaukee River located directly behind the Tilton Leather Company. This dam does not have any use to the Public Service Company of New Hampshire and we have reviewed the use to the industries in the area. We have also discussed the ownership of the dam with the Towns of Tilton and Northfield.

To leave the dam in the river would require a sizeable sum of money for maintenance; therefore, we propose to remove the dam this fall working out with the industries that are involved a connection to the river so that they can take water for processing purposes.

If you have any questions as to the planned action, we will be pleased to hear from you.

Very truly yours,

Eliot Priest Vice President

EP:p

c.c. D. E. Sinville

W. A. Adams, Jr.

L. O. Wilson

#### NEW HAMPSHIRE WATER CONTROL COMMISSION

#### REPORT ON DAM INSPECTION

octed by me on	7/5/ accompan	ied by		
	70			
Spillway Faur				
Gates South Side	e gate in poor a	andition. Till ge in a	mber fram	e to
itle demage o	Counstream. Til	ton side - 1	verside 95th	operation
Other				
E INSPECTIONS	ction			
ES SINCE LAST INSPE E INSPECTIONS This dom (is) (is)	CTION	d lack of		
E INSPECTIONS This dam (is) (is)	CTION  LE  TO a morace because	of lack of		
E INSPECTIONS This dam (is) (is)	CTION  LE  TO a monace because  Levivers edge	of lack of		

Form No. E61A		8442
NEW HAMPSHIRE WA	TER CONTROL COMMISSION	ECORD OF DAM NO. 237.02
Town Tilton	County Merrimack Local	Name
Function of Dam Power	Type Timber.	A Frame
	Sec. BasinWinnipesaukaa Rivar Lo	
Drainage Area, Total i4.0	sq. mi.: Controlledsq. mi.	Net Uncontrolled ill. ill. ill. sq. mi.
Reservoir Area, Full Pond in	acres: At Max Draw	lown acres:
Reservoir Capacity	mcf. ac. ft	net D. A. Francisco, in Total D. A.:
Overall Length of Dam	ft. Max. Depth Water	at Dam ft.
	ft Minimum Freeboa	
Spillway Capacity Third his	cis cfs. per sq. mi.	
Highest Flood Flow of Record	cfs. pe	r sq. mi. Date
Estimated Maximum Probable	Flood	
REMARKS: 15 DOWN STREET		And the second s
Card Prepared by	Checked byAp	proved for File: Date 0/21/41

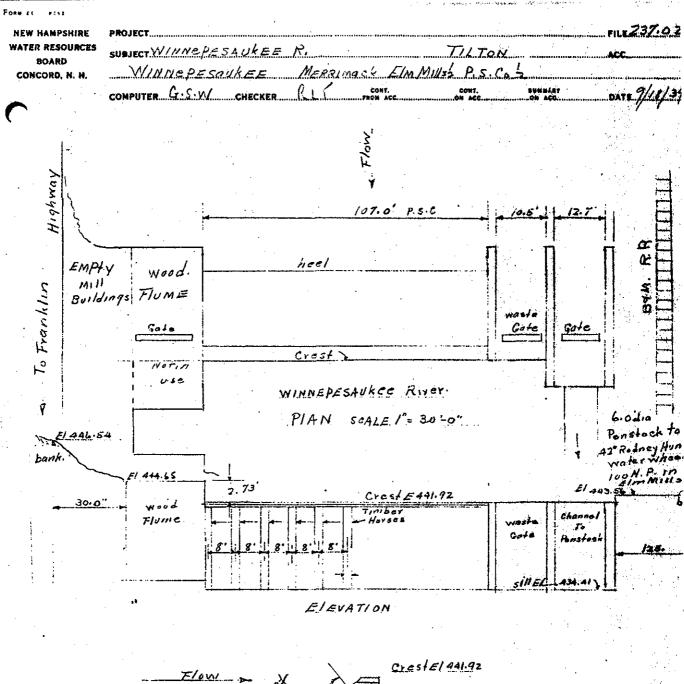
ontractor	::: Address				7	
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Date Office-Routine		Inspec	tion Durin	g Construc	ion Y	
pplication Received	Date	Inspector	Memo	Date	Inspector	Memo
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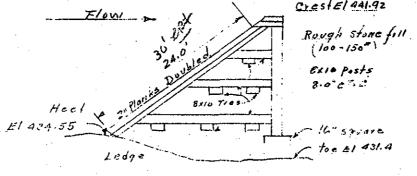
# NEW HAMPSHIRE WATER CONTROL COMMISSION DATA ON DAMS IN NEW HAMPSHIRE

LOCATION	STATE NO237.02.	
Town Tilton	: County Belknap	*****
StreamWinnipesaukee R.		
Basin-Primary Merrimack	: Secondary Winnipesaukee R.	•••
Local Name Upner Dam		. 4-4
Coordinates—Lat. 443 25'1+-9500	Long. 71° 35′4 2°2900	
GENERAL DATA		
	Uncontrolled	
	Construction Prior to 1886 1	
Height: Stream bed to highest elev16	.ft.: Max. Structure	ft
Cost—Dam	: Reservoir	*****
DESCRIPTION Timber on hard pan "A" 1	Frame	• •
Waste Gates		
·	ft. high x ft. v	
	: Total Areasq	-
		,,,
Waste Gates Conduit		
	ials	
	ft.: Area	ı. ft
Embankment		
	. ft.: Min	
• =	Downstream on	
	•	
	: Left of Spillway	******
Spillway  Materials of Construction	Timber	
	ft.: Net107	
Height of narmonant section May (12)	12 ft: Min. 12	II
	: Height	
Floration Dominant Creek 441.92		II
	:cfs/sq. mi.	******
•	cis/sq. mi.	
Abutments Materials	•	
Freehoard May 4	. ft.: Min. "" [] []	£
Headworks to Power Devel.—(See "Data or	·	11
	ic Ser. Co. 🖢 (	
	**************************************	*******
REMARKS Condition poor.	B-16	
Tabulation ByRLT		

# NEW HAMPSHIRE WATER CONTROL COMMISSION DATA ON WATER POWER DEVELOPMENTS IN NEW HAMPSHIRE

	La caracteristic de la car	_		-		.02
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		*******************************	******	******		
ENERAL DATA				•		
		ft.: A				
	· .					
_	·····	ac. ft.: St	orage		*************	, ac
DESCRIPTION	Timber on ha	rd pan	•			
Racks		4		• •		
		M				
	**********************	Sq. Ft.: N	vet	****************	**************	sq
Head Gates					•	
		***************************************			•	
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Size		: Length	*************	,		
i .		: Length		,		
Size	· · · · · · · · · · · · · · · · · · ·	: Length	*************	,		
Size Turbines	Dam No	: Length		· · · · · · · · · · · · · · · · · · ·	•••••••••••••••••••••••••••••••••••••••	
Size Turbines th side of Dam,	Dam No	Length  237.02  ed by Public Ser	rvice, Mill c	losed •10.5 he	ad feet.	
Size Turbines  th side of Dam,	Dam No Tilton side own penstock ,llhe	Length  2. 237.02  2. 237.02  2. 24 by Public Sepand feet.48"Rodne	rvice, Mill c	losed •10.5 he	ad feet.	
Size Turbines  th side of Dam,	Dam No Tilton side own penstock ,llhe	Length  2. 237.02  2. 237.02  2. 24 by Public Sepand feet.48"Rodne	rvice, Mill c	losed •10.5 he	ad feet.	
Size Turbines  th side of Dam,	Dam No Tilton side own penstock ,llhe	Length  2. 237.02  2. 237.02  2. 24 by Public Sepand feet.48"Rodne	rvice, Mill c	losed •10.5 he	ad feet.	
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Size Turbines  th side of Dam,	Dam No Tilton side own penstock ,llhe	Length  2. 237.02  2. 237.02  2. 24 by Public Sepand feet.48"Rodne	rvice, Mill c	losed •10.5 he	ad feet.	
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SECTION

B-18

### NEW HAMPSHIRE WATER RESOURCES BOARD

## INVENTORY OF DAMS AND WATER POWER DEVELOPMENTS

PAM		•	4250
BASIN Merrimack	No. 2	- 427 2	
RIVER Winni pesaukee.	MILES FRO	M MOUTH	- 3895 D.A.SQ.MI. 4/8 (4
OWN	OWNER Elm	MILLS - PUBLICS	ervice Coof A.H.+
OCAL NAME OF DAM	Maner Dam.		
WILE prior to 1866 DESCRIF	TON_"A" Frame	-Timber o	v Hardpan
· · · · · · · · · · · · · · · · · · ·		(Wood Crib	AE)
OND AREA-AGRES	DRAVIDOUM PT.	POED CAPA	CTTY-ACRE FT.
EIGHT-EOP TO BED OF STREET	AM-FD. 16 13.2	J MAX.	MIN.
VERALL LENGTH OF DAM-FT.	/70 MAX.FLOO	HEISHT ABOVE	CREST-FT.
ERMANENT CREST ELEV.U.S.	G.3. 441.92	_ LCCAL GAGE	
AILWATER ELEV.U.S.	G.3.	LOUAL GAGE	
PILLWAY LENGTHS-FT. 50	and 57	FREEBOARD-FT	· 4 (2.73 A)
LASHBOARDS-TYPE, HETGHT ASTE GATES-NO. WIDTH MA	A CHIMITIC FORE	None	ADEGA
			OUEDI
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<i>:5/4</i>			tes from AE.
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NITS NO. HP FEET	FULL GATE		drong Hours But his
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SE PAIRCE PEBLICUT	III y		
EMARKS MILL CLOSED	Head 11.54	04 77/500 510	
	Head 11.54		
EMARKS Mill Closed	Head 11.54		

PATE 8/30/34

Tilton #2 I-3898

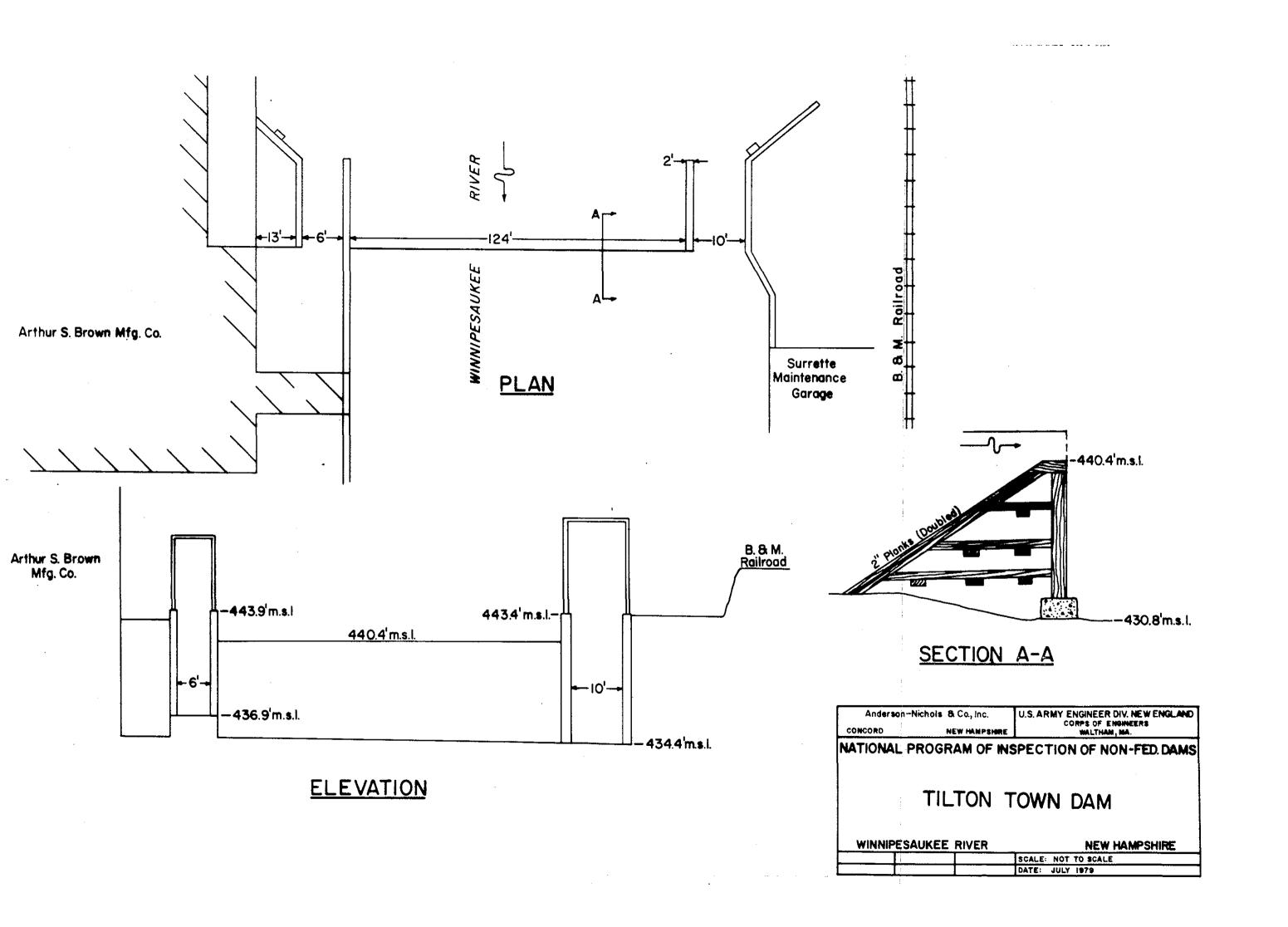
Jinnipesaukee River, Tilton & Northfield, Tilton side, 1/2 dam owned by the Public Service Company of New Hampshire, about 10' head, Northfield side 1/2 dam owned by the Elm Mills Woolen Company about 102 head. Power dam, on basis of 75% - 80%, time - efficiency, 355 H.P., 2533000 Kw-hr. per year; on basis of 90% - 80%, time efficiency, 316 H.P., 2077000 Kw-hrs. per year. As developed, Tilton side not operating. Dam, timber A frame, condition poor, should be repaired, inspected 8-30-34, no record of any previous inspection found, for additional information see I-3893. Flood study not made. The following information was given me as coming from Mr. Harry Daniell Lakeport Dam. His recollection of maximum discharge at the Lake accurred 10 or 12 years ago and reached 1800 cu. ft. over spillway and through gates. The spillway capacity of this dam is greater than 1800 cu. ft.

S. J. Lord

December 17, 1934

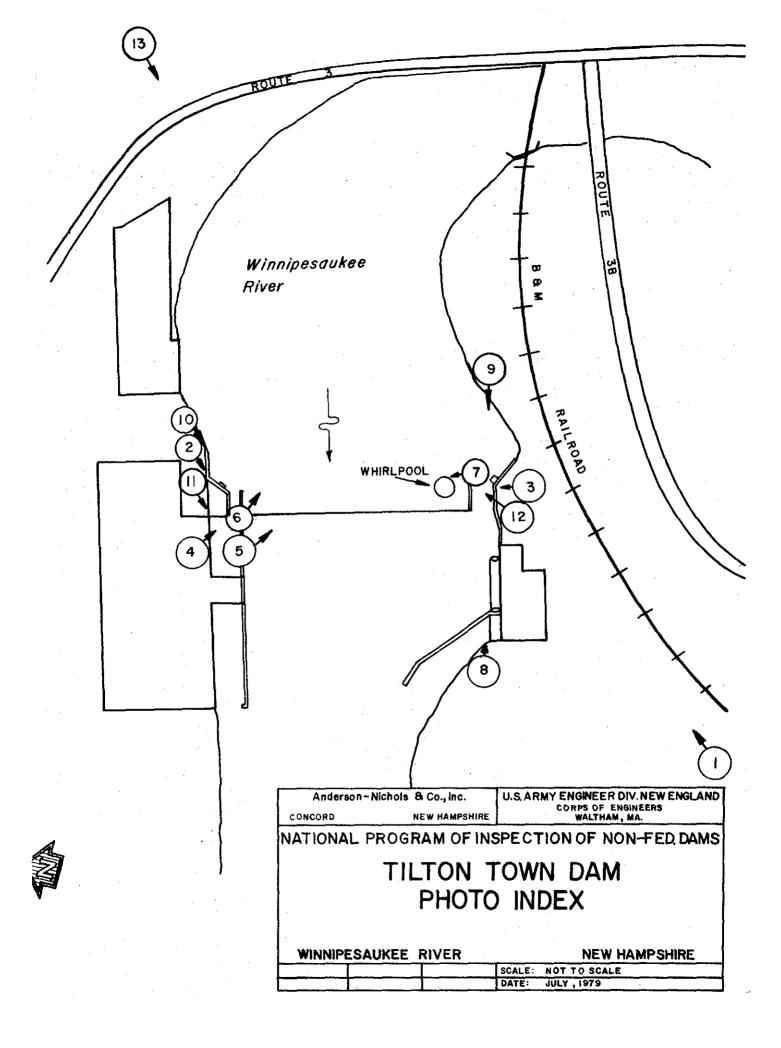
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WHEEL CAP. N. P.	PRIMARY H.	* 80% TIME	WHEEL CAP. H. P.	PRIMARY N. P. BOS TIME
FOR CE	NTRAL STATIONS		EOR ISOLATED	INDUSTRIAL PLANTS
RIVER Winner e	saukee	RATE SEC.		
DRAINAGE AREA	and the second second	Mì. HE	до14 гт.	7.
HOME ADDRESS	and the state of the state of			
NAME OF COMPANY	Flm Mills	Woolen.	Company	
томи ио.1	Town Til to	n. N. H.	<sub>No.</sub> 103b.	PAGE NO.
7010				4

	PUBLIC SERVICE COMMISSION OF NEW HAM	PSHIRE-DAM RECORD	1-3898
	TILTON	town 2	STATE NO.
	STREAM MINNEPESAUREE RIVER		
	DRAINAGE 4/85 QIAMI.	POND AREA	
	TYPE "A" FRAME	NATURE OF HARDPAN	<u> </u>
	MATERIALS OF TIMBER		
	PURPOSE POWER—CONSERVATION—DOMESTIC—REC	REATION-TRANSPORTATION-PUBLIC UT:	LITY Z
	HEIGHTS, TOP OF DAM TO BED OF STREAM	TOP OF DAM TO SPILLWAY CRESTS 4'	
В 1 2	SPILLWAYS, LENGTHS 50' - 57' DEPTHS BELOW TOP OF DAM		of DAM 170
2	FLASHBOARDS NONE		
. •	OPERATING HEAD  CREST TO N. T. W. 74 2 76	TOP OF FLASHBOARDS TO N. T. W.	
	WHEELS, NUMBER 1- Rod ney Hunt 45" 100 H	North good 11H	
	GENERATORS, NUMBER Northern D.C. 30KW KINDS & K.W. 240A ~ 125V		
	H. P. 90 P. C. TIME 100 P. C. EFF.	H. P. 75 P. C. TIME 100 P. C. EFF.	
	REFERENCES, CASES, PLANS, INSPECTIONS.		
	REFERENCES, CASES, PLANS, INSPECTIONS.  REMARKS  OCA Charlo Zillia Z  Non-Tapala Z  Charlo Zillia Z  And Tapala Z		2151
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	any larger and the		



APPENDIX C

PHOTOGRAPHS





April 6, 1979

Figure 2 - Looking south across the upstream face of the dam from the north abutment.



April 6, 1979

Figure 3 - Looking north across the upstream face of the dam from the south abutment.



April 6, 1979
Figure 4 - View of the spillway. Note the uneven level of the water.



April 24, 1979

Figure 5 - Looking at the spillway where local support failures have occurred. Note the water discharging through the spillway through holes in planking.



April 24, 1979

Figure 6 - Closeup of the deteriorated planking on the upspream side of the spillway.



April 24, 1979
Figure 7 - View of the whirlpool located over a hole in the planking.



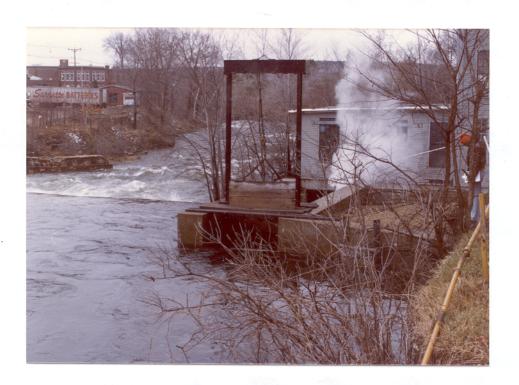
April 6, 1979

Figure 8 - View of the downstream face of the south abutment.



April 6, 1979

Figure 9 - View of the upstream face of the south abutment. Note the concrete box inlet structure.



April 6, 1979 Figure 10 - Looking at the upstream face of the north abutment.



April 6, 1979
Figure 11 - View of the sinkhole observed in the fill at the north abutment.



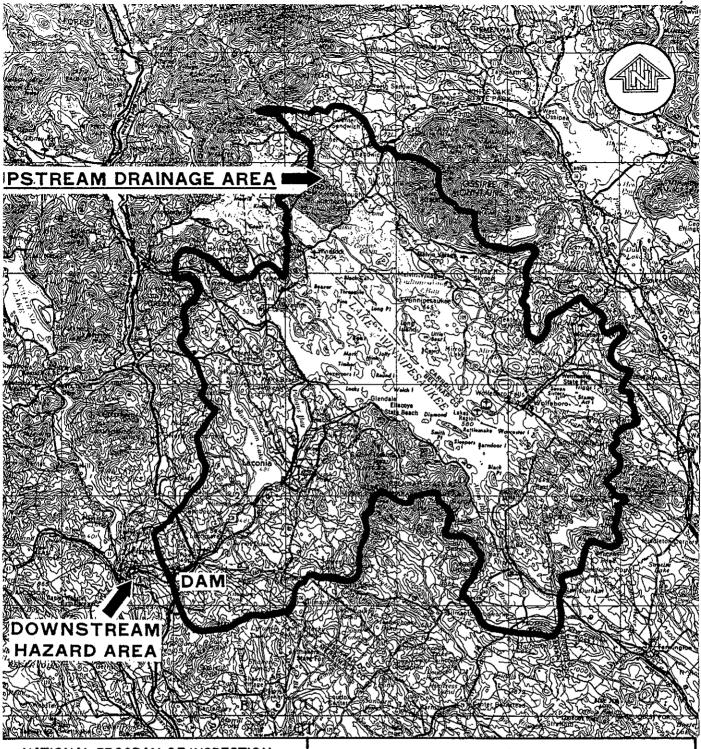
April 6, 1979
Figure 12 - Looking upstream at the north approach channel from the south abutment.



April 1979 Figure 13 - Overview of the downstream channel.

### APPENDIX D

HYDROLOGIC AND HYDRAULIC COMPUTATIONS



NATIONAL PROGRAM OF INSPECTION
OF NON-FED. DAMS
TILTON TOWN DAM
TILTON, NEW HAMPSHIRE
REGIONAL VICINITY MAP

DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASSACHUSETTS

IDERSON-NICHOLS & CO., INC.

CONCORD, N.H.

SCALE IN MILES
5 10

MAP BASED ON U.S.G.S. I:250,000 SERIES TOPOGRAPHIC MAPPING. NK-19-1 PORTLAND ME,NH 1956, REVISED 1972.

# TILTON TOWN DAM HYDROLOGIC HYDRAULIC ANALYSIS

Page 1 of 5 L. Williams 4/20/79

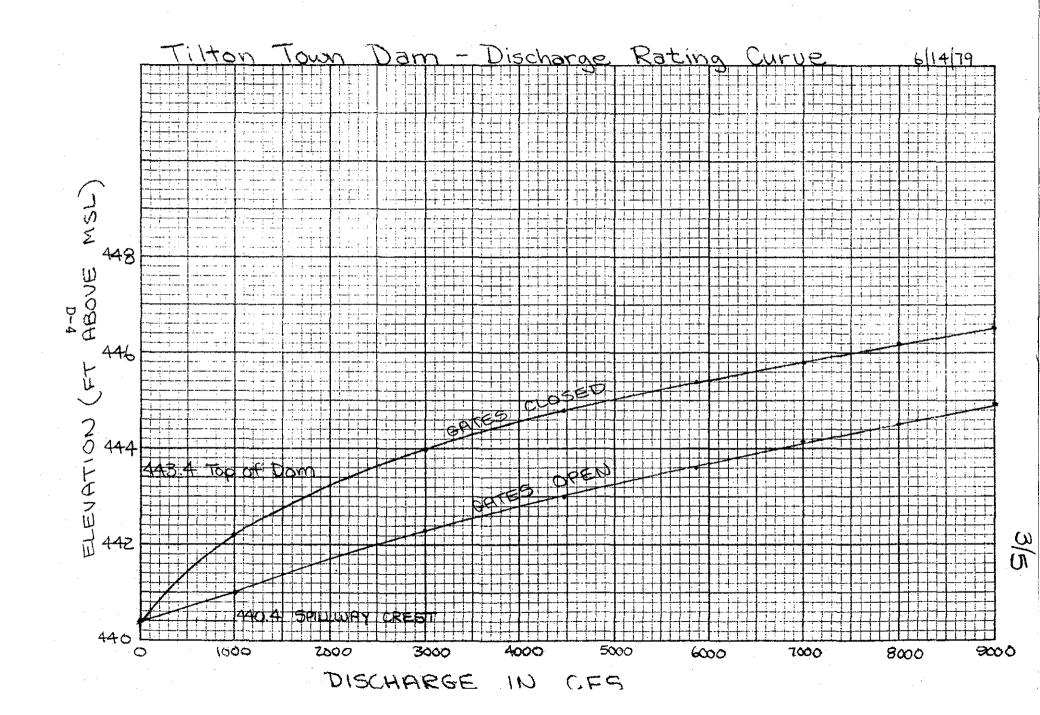
DA 473 mi<sup>2</sup>
Size Classification = Small
Hazard Classification = Significant
Test Flood = 4 PMF to 1/2 PMF
Chosen test flood = 1/2 PMF

Test flood inflow cannot simply be determined by use of the PMF guide curves due to the complexity of the hydrologic and hydraulic conditions which comprise the Winnipesaukee River drainage basin. Flooding on the Winnipesankee River and its associated bays and lakes is to a large extent controlled by Lochmere Dam on Lake Winnisquam, Avery Dam on the Winnipesaukoe River, and Lakeport Dam between Opechee Lake and Paugus Bay. Referring to the Northfield and Tilton Flood Insurance Studies, peak discharges on the Winnipesaulos River were determined at Lakeport Dam, Avery Dam, and Lochmere Dam Using various hydrologic methods. (Note: Northfield FIS, done by ANCO, received backup from Hamilton Engineering who performed the Titton FIS Study, September 1977.) The peak discharge at the Tilton gage during à 500-year storm was determined to be 7,570 cfs, this gage is located 0.4 miles upstream of Tilton Town Dam. The discharge at Lakeport Dam during a flooding event of this magnitude was determined to be 4,300 cfs.

D-2

Water Surface elevations were computed through the use of the Corps of Engineers HEC-2 step-backwater computer program. A subsequent study was performed by ANCO in December 1978 entitled Hydraulic Evaineering Analysis for Evaluating Flood Stage Reduction on the Winnipesantee River, New Hampshire, This study utilized HEC-Z modeling and because this study is more recent and reflects existing conditions on the Winnipesaukee River, this hydraulic model was used in developing a rating curve for Tilton Town Dam. A test file covering the study area was taken from this model and various discharges ranging from 1000 cfs to 9000 cfs were analyzed. From this analysis the following rating curve points we're established, assuming both gates closed: Discharge (cfs) Elevation (ft. above MSL) 440.4 1000 442,24 3000 443,96 4475 444,76 445.38 5875 445.82 7000 0008 446,21 9000 446,54

Using these points a rating curve can be drawn. See page 3.
Another HEC-Z run was made assuming both gates are fully opened. This curve is also plotted on page 3.



Test Flood = 7,570 cfs

With gates closed > 446' MSL

Top of dam > 443.4' MSL

Test Flood would overtop the dam

by Z.6 feet. Spillway capacity @

top of dam is 2200 cfs or 29

percent of test flood with gates closed.

With gates open \$ 444.3' MSL

Top of dam \$ 443.4' MSL

Dam would be overtopped by 0.9 foot.

Spillway capacity including both gates open is 5300 cfs or 70 percent of test flood. Therefore, the combined capacity of both gates \$ 3100 cfs.

Spillway Capacity @ test flood elevation of 446' MSL

Q = CLH<sup>3/2</sup> 2200 cfs = C·124·3.0<sup>3/2</sup> (From HEC-Z run) 3.41 = C

 $Q = 3.41 \cdot 124 \cdot 5.6^{32}$ = 5603.5 cfs  $\frac{5}{5}$  5605 cfs

Following is the HEC-Z input and summary list for a test file run under conditions of closed gates and open gates. (See pages D-7 > D-16.)

## BREACH ANALYSIS - TILTON TOWN DAM

Determine downstream hazard if breach were to occur @ top of dam - 443.4' MSL

Qp, = 27 WbVg yo3/2 where: Wb=breach width

g = 32.2 ft/sec2

Yo = pool elev - U/s river bed

Wb = 172 × 0.40 = 69 feet

Yo = 443.4 - 434 = 9.4 feet

Qp, = 3343 cfs

Qpz = Q going over dam other than at breach

Q = CLH3/2

Therefore consider the constrainty of the constrainty of

=972 cfs Assume gates closed. Qp3 = total breach = 4315 cfs

= 3.4.55,312

this flow is similar to the 4475 cfs used in Reference 5 (1978 ANCO study). Therefore, this profile could be utilized to estimate the level of probable damages due to dam failure under the above conditions. Elevations of potential damage points were obtained for use in the ANCO study. Looking at this profile it can be seen that the only damage caused by a breach of Titton Town Dam would be to the Arthur S. Brown Mfg. Co. building. A portion of this building is located in the channel immediately downstream of the dam and is the working area for plants which utilize the pondage for plants which utilize the pondage for damage could be appreciable. The pondage also supplies water to an auxiliary fire pump. Therefore, Tilton Town Dam was classified - SIGNIFICANT HAZARD.

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		21.000	4.000	22.000	54.000	49.000	34.000	17.000	0.0	0.0	0.0			
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(	SR SR	410.800 402.100	9489.000 9730.000			55.000 12.000	408.400 398.300	9686.000 9849.000	407.00 396.80			402.300 395.000	9712.000 9910.000	
	3R <b>3R</b> -	394,300 405,100	9934,000 10142,000			64+000 68+000	402.000 420.200	9981.000	403.10	0 10022. 0.	000	103.700 0.0	10075.000	
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	- GK	405.300	A281.000	403.100	9899.000	402.700	~~_9 <i>7</i> 16.7000 ~	403.900	9734.000	403.500	~~9741.000~	
	GR	402.000	9788.000	400.800	9822.000	401.300	9864,000	402,000	9899.000	405.300	9921.000	
	GR	408,400	9935.000	411.400	9958.000	423.100	9987.000	0.0	0.0	0.0	0.0	
7							·	•	•			
•												
	X1	21425,000	38.000	9864.000	10000.000	700.000	1680.000	910.000	0.0	0.0	0.0	
	GK	420,300	8700.000	417.400	8712.000	413.400	8727.000	414.600	8792.000	415.300	8853.000	
	ĞŔ	413,300	8872.000	413.200	8945.000	411.900	8965.000	413.000	9043.000	417.700	9131.000	
_	GR	417.700	9267.000	417,900	9363,000	416,700	9450.000	416.400	9523.000	414.600	9610,000	
	GR	413,200	9643.000	414.900	9738.000	415.100	9833.000	413,200	9858.000	407.600	9864,000	
	GR	405,600	9876.000	401.800	9889.000	400.400	9913.000	401.100	9931.000	402,200	9949.000	
	GR	402,900	9962.000	405.600	9996.000	407.000	10000,000	408.700	10010.000	408.800	10025.000	
	GR	408.000	10032.000	412.200	10111.000	412.700	10160.000	416.100	10263.000	414.200	10275.000	
	GK	418.000	10357.000	419.000	10365.000	423.000	10375.000	0.0	0.0	0.0	0.0	
	Xi	21965.000	21.000	848,000	1002.000	250.000	440,000	540,000				
	GR	419.700	375.000	417.300	450.000				0.0	0.0	0.0	
-4-						415.200	483.000	416.000	545.000	416,100	593.000	
•	GR GR	415.800 406.000	645,000	417,500 404,000	704.000 8 <i>7</i> 5.000	416.200	813.000	413.600	848.000	411.200	853.000	
	GR	401.200	866.000	402,400		404.600	888.000	403.800	903.000	401.200	934.000	
	GR	424.600	960,000 1015.000	0.0	973.000 0.0	406.000	988.000	411.800	1000.000	413.700	1002,000	
	NC	0.100	0,110	0.035	0.300	0.0	0.0	0.0	0.0	0.0	0.0	
	110	0.100	0,110	0.033	0.300	0.000	0.0	0.0	0.0	0.0	0.0	
_												
	X1	23220.000	20.000	783.000	990,000	1000.000	880.000	1255,000	0.0	0.0	0.0	
	GR	419.800	776.000	415.600	783,000	408.100	796.000	407,000	799.000			
<b></b> .	GR	403.500	847.000	401.000	876.000	402.600	904.000	403.600	934.000	404.700 404. <b>900</b>	819.000 951.000	
	GR	407.000	966,000	414.000	990.000	414.400	1000.000	414.500	1084.000	414.400	1183.000	
	GR	414,500	1265.000	417.400	1335.000	415.200	1438.000	413.200	1468.000	419.800	1555.000	
igas	. D.\		1100.000	11,1,00	1555,000	4131200	14301000	413.200	1400.000	417.600	13331000	
н								* *				
Þ	X1	25615.000	19.000	884.000	990.000	2600.000	720.000	2395.000	0.0	0.0	0.0	
œ	GR	420.900	871.000	410.000	884.000	408.700	887,000	407.200	893.000	404.800	907.000	
	GR	405,100	917.000	404.800	935.000	405.800	950.000	407.300	967.000	408.700	975.000	
	GR	410.300	990.000	411.200	1000.000	413.200	1055.000	413.200	1092.000	411.500	1105.000	
· -	GR	413.100	1159.000	415.700	1167.000	419.900	1185.000	420,900	1197.000	0.0	0.0	
	NC	0.085	0.110	0.040	0.300	0.500	0.0	0.0	0.0	0.0	0.0	
		•									- • -	
	•- •											
	X1	26545.000	31.000	886,000	992.000	600.000	480.000	930.000	0.0	0.0	0.0	
	GR	434.700	752,000	415.400	788.000	415,400	811.000	414.200	830.000	414.600	869.000	
r <b>it</b>	GR	412,900	886,000	411.300	889.000	408.200	900.000	408.800	918.000	409.800	933.000	
	GR	409,700	957.000	409.700	965,000	410.500	987,000	412.100	988,000	413.400	992.000	
	GR	416,800	995.000	417.100	1000.000	419.000	1052.000	418.600	1066.000	417.300	1093.000	
_	GR	416.900	1116.000	420.100	1144.000	422.900	1201.000	421.600	1243.000	423,200	1283.000	
	GR	423.200	1291.000	420.400	1395.000	420+500	1531,000	419.900	1643.000	418.400	1656.000	
	GR	424.800	1683.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
_	NC	0.0	0.085	0.0	0.300	0.500	0.0	0.0	0.0	0.0	0.0	
					*							
											* .	
	X1	27595,000	14.000	855.000	1000.000	800.000	1320,000	1050.000	0.0	0.0	0.0	
	GR	440.600	816.000	440.200	848.000	434.500	855.000	423.500	870.000	421.300	890.000	
4	GR	421.100	899.000	420.800	918.000	421.700	946.000	421.600	958.000	423.000	971.000	
•	GR	423,500	974.000	434.200	1000.000	437 • 400	1114.000	443,400	1166.000	0.0	0.0	
						•						
		· MMM44 4	40 4								•	
	XI	77910.000	19.000	1000.000	1218.000	265.000	350.000	315.000	0.0	0.0	0.0	
	GR	449.200	868.000	447.600	872.000	445.700	934.000	444.200	1000,000	432.000	1020.000	
	GR	428.500	1032.000	427.200	1042.000	427.200	1051.000	427.800	1072.000	427.200	1085.000	
	GR	426.300	1095.000	425.800	1131.000	426.500	1167.000	428.700	1181.000	437.600	1190.000	
	GR	439.000	1209.000	442,900	1218.000	443,100	1233.000	449.200	1248.000	0.0	0.0	•
_	NC	0.0	0.0	0.0	0.300	0.500	0.0	0.0	0.0	0.0	0.0	
							•					
	v.	20010 000	17 000	4040							_	
· 	X1	28010.000	13.000	1048.000	1196.000	85.000	130,000	100,000	0.0	0.0	0.0	

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	·	TGR-	753,300	780.000	- 452.400	1000	-000	444.400	1010.000	443,800	1048.00	76		048.100		~···
-		GR	430.800	1195.900	443.200	1196		444.100	1208.000	446.500	1208.10			216.000		
		GR SB	439,800	1216.100	439.400			450.000	1228.100	0.0	0.0		.0	0.0		
	TILTON	32	0,900	10.000	3.000	0	•0	0.100	0.010	0.010	0.0	434	400	430.800		,
	كَ	1														
	ģ	X1 X2	28015.000	17.000	932.000			5.000	5.000	5.000	0.0	0	.0	0.0		
			0.0 17.000	0.0 864.000	1,000 453,300		•400 •0	440.400 884.000	0.0	0.0	0.0		•0	0.0		
	ä	BT	932.000	443.400			.100	442.900	452.400 434.500	0.0 942.000	894.00 442.90		.400 .500	0.0 942.100		
	נושסד	BT	443.400	0.0	944.000	443	.400	0.0	944.100	440.400	0.0	1068		440.400		
	۶. خ	BT	0.0 1076.000	1068,100 442,400	443,900			1070.000 443.900	443.900	0.0	1070.10			437.000		
		1 to T	450.000	0.0	0.0		.0	0.0	0.0	1108.000	440.00		.0 1	.108.100 0.0		
	DAM	GR	453.300	864.000	452,400		.000	444.400	894.000	443,400	932.00			932,100	•	
	3	GR GR	434.400 443.900	942.000 1068.100	443.400 443.900		100	443.400	944-000	440.400	944.10	0 440	400 1	.068.000		
		GR	440.000	1108.000	450.000			436.900 0.0	1070.100 0.0	436.900 0.0	1076.00		.900 1 .0	.074 <b>.1</b> 00		
	9	NC	0.0	0.0	0.0		.300	0.500	0.0	0.0	0.0		•0	0.0		
		Χí	28027.000	17.000	1006.000	1173	.000	12,000	12.000	12.000	0.0	7	.000	0.0		
		GR	453.100	965,000	452,200	984	.000	449.600	1000.000	449.000	1006.00			025.000		
	-	GR GR	440.800 432.100	1027.000	437.300 435.300			436.800	1063.000	433.800	1086.00	0 434	.800 1	103.000		
		GR	447.400	1173.100	448.100			434.800 0.0	1164.000 0.0	437.700 0.0	1167,00		.600 1 .0	173.000		
	**								0.0	<b>0.0</b>	V.0	0	• • • • • • • • • • • • • • • • • • • •	0.0		
		X1	28071.000	0.0	0.0	0	.0	60,000	70+000	44.000	0.0		000			
	1.5	ИC	0.080	0.0	0.035		.300	0.500	0.0	0.0	0.0	0.	.000	0.0	•	
	•															
	D	Х1	28525.000	25,000	1051.000	1179	.000	360.000	520.000	454.000	0.0	0	. 0	0.0		
	9	GR	465.900	521.000	463,700	587	.000	462.800	619.000	461.500	666.00			696.000		
**	-	GR	455.800 450.600	815.000 1003.000	451.700 441.800		.000	451.400	967.000	451.800	982.00		.100 1	.000.000		
•	:	GR	440.900	1063.000	438.800			441.600 433.900	1025.000 1094.000	443.000 436.200	1033.00			051.000 140.000		
		GR:	435.900	1164.000	440,900	1174	.000	445.000	1179.000	446.600	1200.00			206.000	•	
	•	EJ	0+0	0.0	0.0	0	•0	0.0	0.0	0.0	0.0	Ο.	.0	0.0		
		? 1	1842/1 unn													
	•	SUM	MARY PRINTO	FUT		-							-	•		
		٠								•						
	<del>-</del> -		SECNO	XLCH	ELTRD	ELLC	CWSEL	XLBEL	ELMIN	RBEL	VCH	Q	OLOB	OCH	QROB	
			14790.000	220.00	0.0	0.0	400.97	7 405.20	394.50	405.10	1.96	1000.00	0.0	1000.00	0.0	
	<b></b> .		14790.000	220.00	0.0	0.0	405.16	405.20		405.10	2.74	3000.00	0.0	3000.00	0.00	
			14790.000 14790.000	220.00 220.00	0.0	0.0	407.03			405.10	3.22	4475.00	3.57	4468.82	2.61	
			14790.000	220.00	0.0	0.0	408.57			405.10 405.10	3.57 3.80	5875.00 7000.00	31.26		12.48	
			14790.000	220.00	0.0	0.0	410.5	405.20		405.10	3,99	8000.00	101,72 193.78	6872.66	25.61 40.76	
	_		14790.000	220.00	0.0	0.0	411.3	3 405.20	394.50	405.10	4.15	9000.00	314.35		61.50	
	<b>†</b>		16245.000	1455.00	0.0	0.0	401.33	402.00	394.30	402.00	1.43	1000.00	0.0	1000.00	0.0	
	•		16245.000	1455.00	0.0	0.0	405.5	402.00	394.30	402.00	1.96	3000.00	150.74		96.01	
	=		16245.000 16245.000	1455.00 1455.00	0.0	0.0	407.41			402.00	2.24	4475.00	320,29		287.79	
	1		16245.000	1455.00	0.0	0.0	410.0			402.00 402.00	2.44	5875.00 7000.00	518,24 725,22		506.69 689.72	
	-		16245.000	1455.00	0.0	0.0	410.97	7 402.00	394.30	402.00	2.67	8000.00	931.60		857.79	
			13245.000	1455.00	0.0	0.0	411.8	402.00	394.30	402.00	2.76	9000.00	1159.11		102B.32	
	-		18030.000	1785.00	0.0	0.0	403.14	4 404.00	399.60	404.00	3.70	1000.00	0.0	1000.00	0.0	1.
			18030.000 18030.000	1785.00	0.0	0.0	406.54	404.00	399.60	404.00	4.84	3000.00	30,28	2948.82	20.89	
	,		18030.000	17 <b>85.0</b> 0 1785.00	0.0	0.0	408.32			404.00 404.00	5.49 5.97	4475.00	92.38		66.96	
											U•7/	5875.00	170.11	5577.41	127.48	

				1.1		1								
	18030.000	1785.00			410.84	404.00	05: <del>99: 50</del>	404.00	~~~~	- 7000 <del>. 0</del> 0	247.7/	3531.96	190:27	
	18030.000	1785.00	0.0	0.0	411.69	404.00	399.60	404.00	6.60	8000.00	328.97	7416.08	254.95	
	18030.000	1785.00	0.0	0.0	412.50	404.00	399.60	404.00	6.84	9000.00	423.96	8247.28	328.76	
			• • •	,		1000	0.,,,,	104100	3.0	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	723476	024/120	340+/0	
	18620.000	570.00	0.0	0.0	404.64	409.00	400.00	409.20	4.09	1000.00	0.0	1000.00	0.0	
	18620.000	590.00	0.0	0.0	407.52	409.00	400.00	409.20	4.63	3000.00	0.0	3000,00		
*	18620.000	590.00	0.0	0.0	409.23	409.00	400.00	409.20	4.60	4475.00	0.03		0.0	
	18620.000	590.00	0.0	0.0	410.66	409.00	400.00	409.20		5875.00		4474.97	0.00	
	18620.000	590.00	0.0	0.0					4.63		4.48	5863.09	7.43	
	19620.000				411.71	409.00	400.00	409.20	4.70	7000.00	15.50	6952.74	31.76	
	18620.000	590.00 590.00	0.0	0.0	412.57	409.00	400.00	409.20	4.77	8000.00	32.57	7903.73	63.70	
	18620.000	390.00	0.0	0.0	413.39	409.00	400.00	409.20	4.85	9000.00	52.65	8844.77	102.58	
<b>~</b> .	20515.000	1895,00	0.0	0.0	405.51	411 00	400 00	*** **	. 70	1000 00				
	20515.000	1875.00	0.0	0.0	408.46	411.00 411.00	400.80 400.80	411.40 411.40	1.32 1.97	1000.00 3000.00	0.0	1000.00	0.0	
	20515.000	1875.00	0.0	0.0	410.12	411.00					0.0	3000.00	0.0	-
	20515.000						400.80	411.40	2.23	4475.00	0.01	–	0.0	
	20515.000	1895.00 1895.00	0.0	0.0	411.50	411.00	400.80	411.40	2.40	5875.00	16.86	5858-14	0.00	
			0.0	0.0	412.50	411.00	400.80	411.40	2,50	7000.00	108.62	6891.23	0.15	
	20515.000	1875.00	0.0	0.0	413.33	411.00	400.80	411.40	2.56	8000.00	280.23	7719.11	0.66	-
•	20515.000	1875.00	.0.0	0.0	414.12	411.00	400.80	411.40	2,59	9000.00	518.85	3479.54	1.60	
•							•						*	
	SECNO	XLCH	E. TOT	EL 1.0	CHOC:	V. 80.		nne:		_				
	SECHU	ALGH	ELTRD	ELLC	CWSEL	XLBEL	ELMIN	RBEL	ACH	Q	GLOB	<b>QCH</b>	QROB	
	21425.000	910.00	0.0	0.0	405.79	407.60	400.40	407.00	2.50	1000.00		1000 00		
•	21425.000	910.00	0.0	0.0	408.71						0.0	1000.00	0.0	
	21425.000	910.00	0.0			407.60	400.40	407.00	3.82	3000.00	0.12	2996.22	3.66	
	21425.000			0.0	410.34	407.60	400.40	407.00	4.40	4475.00	1.28	4424.37	49.35	
		910.00	0.0	0.0	411.68	407.60	400.40	407.00	4.83	5875.00	3.64	5738,75	132.61	
	21425.000	910.00	0.0	0.0	412.66	407.60	400.40	407.00	5.12	7000.00	10.33	6761.70	227.97	
	21425.000	910.00	0.0	0.0	413.46	407.60	400.40	407.00	5.32	8000.00	40.98	7616,26	342.76	
4 (	21425.000	910.00	0.0	0.0	414.24	407.60	400.40	407.00	5.46	9000.00	126.82	8395.26	477.92	
	21965,000	540.00	0.0	0.0	406.09	417 (0	404 00	447 70	0.44	4000 00	• •			
175	21965.000	540.00	0.0	0.0	409.04	413.60 413.60	401,20	413.70	2,61	1000.00	0.0	1000.00	0.0	
Ų.	21965.000	540.00	0.0	0.0			401.20	413.70	3.92	3000.00	0.0	3000.00	0.0	
	21765,000	540.00	0.0		410.65	413.60	401.20	413.70	4+52	4475.00	0.0	4475.00	0.0	
0				0.0	411.99	413.60	401.20	413.70	4.96	5875.00	0.0	5875.00	0.0	
_	21965.000	540.00	0.0	0.0	412.95	413.60	401.20	413.70	5.26	7000.00	0.0	7000.00	0.0	
	21965.000	540.00	0.0	0.0	413.74	413.60	401.20	413.70	5.51	8000.00	0.01	7999.99	0.00	
	21965.000	540.00	0.0	0.0	414.50	413.60	401.20	413.70	5.74	9000.00	1.07	8998.87	0.06	
	23220.000	1255.00	0.0	0.0	406.77	445 (0	404.00	414.00						
	23220.000	1255.00	0.0	0.0	409.83	415.60 415.60	401.00 401.00	414.00	1.89	1000.00	0.0	1000.00	0.0	
	23220.000	1255.00	0.0		411.50				2.82	3000.00	0.0	3000.00	0.0	
ş	23220.000	1255.00		0.0		415-60	401.00	414.00	3+25	4475.00	0.0	4475.00	0.0	
	23220.000	1255.00	0.0	0.0	412.88	415.60	401.00	414.00	3.57	5875.00	0.0	5875.00	0.0	
	23220.000	1255.00			413.87	415.60	401.00	414.00	3.80	7000.00	0.0	6999.14	0.86	
	23220.000		0.0	. 0.0	414.68	415+60	401.00	414.00	3.97	8000.00	0.0	7985,40	14.59	
	23220.000	1255.00	0.0	0.0	415.45	415,60	401.00	414.00	4.10	9000.00	0.0	8878.31	101.70	
	25615.000	2395.00	0.0	0.0	409.15	410.00	404.80	410.30	3.56	1000.00		1000 00		
·	25615.000	2395.00	0.0	0.0	412.04	410.00					0.0	1000.00	0.0	7
	25615.000	2375.00	0.0	0.0	413.55	410.00	404.80	410.30	5.14	3000.00	1.23	2986.49	12.29	
	25615.000	2375.00					404.80	410.30	5.86	4475.00	5.21	4342.39	127.40	
	25615.000		0.0	0.0	414.77	410.00	404.80	410.30	. 6.31	5875.00	11.09	5491.60	372.31	
	25615.000	2395.00	0.0	0.0	415.67	410.00	404.80	410.30	6.60	7000.00	17.14	6371.37	611.49	
		2395.00	0.0	0.0	416.41	410.00	404.80	410.30	6.84	8000.00	23.36	7134.20	842.44	
<del></del>	25615.000	2395.00	0.0	0.0	417.10	410.00	404.80	410.30	7.06	9000.00	30.31	7884.79	1084.90	
-	26545.000	930.00		^ ^	444 00	440.00		*** **		4000 00				
	26545.000	930.00	0.0	0.0	411.90	412.90	408+20	413.40	4.28	1000.00	0.0	1000.00	0.0	
	26545.000			0.0	414.35	412.90	408.20	413.40	6.11	3000.00	9.16	2990.70	0.15	
		930.00	0.0	0.0	415.65	412.90	408,20	413.40	6.94	4475.00	123.76	4349.81	1.43	
	26545.000	930.00	0.0	0.0	416.69	412.90	408.20	413.40	7.51	5875.00	341.56	5529.61	3.83	
	26545.000	930.00	0.0	0.0	417.46	412.90	408.20	413.40	7.86	7000+00	555.05	6432.73	12.22	
	26545.000	930.00	0.0	0.0	418.10	412.90	408,20	413.40	8.12	8000.00	759.47	7196.70	43.83	
•	26545.000	930.00	0.0	0.0	418.71	412.90	408.20	413,40	8.33	9000.00	971.74	7925.23	103.03	
	MATERIA AAA	1080 00		<b>.</b> .	.==									
	27595.000 27595.000	1050,00	0.0	0.0	423.10	434.50	420.80	434.20	6.56	1000.00	0.0	1000.00	0.0	
		1050.00	0.0	0.0	424-61	434.50	420.80	434.20	9.66	3000.00	0.0	3000.00	0.0	
	<b>*</b> 27595.000	1050.00	0.0	0.0	425.50	434.50	420.80	434.20	10.95	4475.00	0.0	4475.00	0.0	

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	<b>*</b> 27595.000	1050.00	7.0	0.0	426.26	434.50	420.80	434.20	11.88	5875.00	<del></del>	5875.00	1 0.0 · · · ·
	* 27595.000	1050.00	0.0	0.0	426.83	434.50	420.80	434.20	12.51	7000.00	0.0	7000.00	0.0
	<b>*</b> 27595.000	1050.00	0.0	0.0	427.29	434.50	420.80	434.20	13.02	8000.00	0.0	8000.00	0.0
	¥ 27595.000	1050.00	0.0	0.0	427.74	434,50	420.80	434.20	13.47	9000.00	0.0	9000.00	0.0
								,,	20777	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	,	7000100	0.0
	27910.000	315.00	0.0	0.0	428.07	444.20	425.80	442.90	5.17	1000.00	0.0	1000.00	0.0
<b>€</b>	27910.000	315.00	0.0	0.0	429.52	444.20	425.80	442.90	7.36	3000.00	0.0	3000.00	0.0
	27910.000	315.00	0.0	0.0	430.34	444.20	425.80	442.90	8.35	4475.00	0.0	4475.00	0.0
	27910.000	315.00	0.0	0.0	431.06	444.20	425.80	442.90	9.04	5875.00	0.0	5875.00	
	27910.000	315.00	0.0	0.0	431.60	444.20	425.80	442.90	9.48	7000.00	0.0		0.0
	27910.000	315.00	0.0	0.0	432.08	444.20	425.80	442.90	9.80			7000.00	0.0
	27910.000	315.00	0.0	0.0	432.53	444.20	425.80			8000.00	0.0	8000.00	0.0
-		. 010700	0.0	V.0	732133	444.20	423+60	442.90	10.10	9000.00	0.0	9000.00	0.0
	SECNO	XLCH	ELTRU	ELLC	CWSEL	XFREF	ELMIN	RBEL	VCH	Q	GLOB	OCH	OROR
	* 28010.000	100.00	0.0	0.0	431.92	443.60	430.80	443.20	6.05	1000.00	0.0	1000 00	
	* 28010.000	100.00	0.0	0.0								1000.00	0.0
•	* 28010.000	100.00	0.0	0.0	433.13	443.60	430.80	443.20	8,70	3000+00	0.0	3000.00	0.0
	* 28010.000	100.00	0.0	0.0	433+85	443.60	430.80	443.20	9.94	4475.00	0.0	4475.00	0.0
	* 28010.000				434.45	443.60	430.80	443+20	10.90	5875.00	0.0	5875.00	0.0
		100.00	0.0	0.0	434.90	443.60	430.80	443.20	11.55	7000.00	0.0	7000.00	0.0
	* 28010.000 * 28010.000	100.00	0.0	0.0	435.30	443.60	430.80	443.20	12.04	8000.00	0.0	8000.00	0.0
	* 20010.000	100.00	0.0	0.0	435.66	443.60	430.80	443.20	12.53	9000.00	0.0	9000.00	0.0
<b></b> .	28015.000	· = ^^	440 40	474 40					_ :_ :_			•	
	S 28015.000	5.00	440.40	434.40	442.10	443.40	434.40	443.90	3.08	1000.00	0.0	982.34	17.66
	28015.000	5.00	440.40	434.40	443.65	443.40	434.40	443.90	5.40	3000.00	0.38	2900.30	99.32
		5.00	440+40	434.40	444.30	443.40	434.40	443.90	. 6.78	4475.00	12.81	4277.59	184.60
	28015.000	5.00	440.40	434.40	444.83	443.40	434.40	443.90	7.84	5875.00	51.00	5546+42	277.58
	71 20013+000	5.00	440,40	434,40	445.21	443.40	434.40	443,90	8.59	7000.00	94.41	6549,54	356.05
41	28015.000 28015.000	5.00	440.40	434.40	445.58	443.40	434.40	443.90	9.13	8000.00	143.23	7426,55	430.22
	28015.000	5.00	440:40	434.40	445.86	443.40	434.40	443.90	9.73	9000.00	191.40	8306.23	502.37
U	~												
P	28027.000	12.00	0.0	0.0	442.24	452.00	435.10	447.60	1.75	1000.00	0.0	1000.00	0.0
$\vdash$	28027,000	12,00	0.0	0.0	443.96	452.00	435.10	447.60	3.68	3000.00	0.0	3000.00	0.0
ì	28027.000	12.00	0.0	0.0	444.76	452.00	435.10	447.60	4.82	4475.00	0.0	4475.00	0.0
_	28027.000	12.00	0.0	0.0	445.38	452.00	435.10	447.60	5.76	5875.00	0.0	5875.00	0.0
	28027.000	12.00	0.0	0.0	445.82	452.00	435.10	447.60	6.45	7000.00	0.0	7000.00	0.0
	28027.000	12.00	0.0	0.0	446.21	452.00	435.10	447.60	6.99	8000.00	0.0	8000.00	0.0
	28027.000	12.00	0.0	0.0	446.54	452.00	435.10	447.60	7.55	9000.00	0.0	9000.00	0.0
	28071.000	44.00	0.0	0.0	442.28	449.00	432.10	444.60	0.99	4000 00		4555 65	
	28071.000	44.00	0.0							1000.00	0.0	1000.00	0.0
<b>♦</b> •	28071.000	44.00		0.0	444.14	449.00	432.10	444+60	2.34	3000.00	0.0	3000.00	0.0
-	28071.000	44.00	0.0	0.0	445.05	449.00	432.10	444.60	3.14	4475.00	0.0	4475.00	0.00
	28071.000		0.0	0.0	445.79	449.00	432.10	444.60	3.81	5875.00	0.0	5875.00	0.00
		44.00	0.0	0.0	446.33	449.00	432.10	444.60	4.30	7000.00	0.0	7000+00	0.00
	28071.000	44.00	0.0	0.0	446+80	449.00	432.10	444.60	4.70	8000.00	0.0	8000.00	0.00
	28071.000	44.00	0.0	0.0	447,22	449.00	432.10	444.60	5.08	9000.00	0.0	8999•99	0.01
<del></del> -	28525.000	454.00	0.0	0.0	442.31	443.20	433.90	445.00	1.44	1000.00	A 70	555	
	28525.000	454.00	0.0	0.0	444.26	443.20	433.90	445.00	3.18	3000.00	0.78	999+22	0.0
	28525.000	454.00	0.0	0.0	445.22	443.20					32.19	2967-81	0.0
<del></del> -	28525.000	454.00	0.0		446.01		433.90	445.00	4.15	4475.00	81.82	4393.14	0.03
	28525.000	454.00	0.0	0.0		443.20	433.90	445.00	4.95	5875.00	142.08	5730 - 89	2.03
ı	28525.000			0.0	446.58	443.20	433.90	445.00	5.51	7000.00	197.77	6795.02	7.21
4-	28525.000	454.00	0.0	0.0	447.08	443.20	433.90	445.00	5.96	8000.00	252.86	7730.07	17.07
	20020+000	454.00	0.0	0.0	447.53	443.20	433.90	445.00	6.40	9000+00	310.27	8660.15	29.58
,													

Tilton Town Dam Test File Both Gates Open

TI CORPS OF ENGINEERS NEW ENGLAND DIVISION-TILTON TOWN DAM

T2 ANDERSON-NICHOLS & CO. INC.
T3 WINNIPESAUKEE RIVER RATING CURVE DATA

	13	WINNIFESA	UKEE RIVER	RATING CUR	VE DATA								
4	Ji	ICHECK	IND	VNIN	IDIR	STRT	METRIC	нчінѕ	O	WSEL	FQ		
		-1.	2.	0.	0. 0	.000320	0.0	0.0	0.	404.000	0.0		
	J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE	•	
~		1.000	0.0	-1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
	73	VARIABLE	CODES FOR S	SUMMARY PRI	TUOTK								
_		38.000	39.000	40.000	41.000	1.000	23,000	42.000	24,000	26.000	43.000	•	
.4.4 U		13.000	14.000	15.000	0.0	38.000	1.000	50.000	61.000	51.000	53.000		
) <del>-</del> 12		21.000	4.000	22.000	54.000	49.000	34.000	17.000	0.0	0.0	0.0		
<b>N</b>	J6	IHLEQ	ICOPY	•				2					
,		1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
-	QT NC	7.000 0.090				5.000 0.300	5875.000 0.500	7000.000	8000.00	0 9000.0	0.0	0.0	
ŧ												-	
3	X1	14790.000				0.000	150.000	250.000	220.00		0.0	0.0	
<b></b> -	GR GR	421.500 405.200				7.000	411.800	620.000	407.90			821.000	
	GR	396.100				7.000 2.000	397,500 398,400	868,000 973,000	396.40 401.50			910.000	
<b>,</b>	GR	411.000				1.000	421.500	1049.000	0.0	0.0		0.0	
}	X1	16245.000	23.000	9812.00	n 000	L.000	1320.000	1520,000	1455.00	0 0.0	0.0	0.0	
	GR	422.600				3.000	414.800	9193.000	415.10			9428+000	
)	GR	410.800	9489.000			5.000	408.400	9686,000	407.00			9712.000	
+	GR	402.100				2.000	398.300	9849.000	396.80			9910.000	
e	GR GR	394.300				4.000	402,000	9981.000	403.10			10075.000	
}	UK	405.100	10142.000	410.00	0 1016	3.000	420.200	10183.000	0.0	0.0	0.0	0.0	
		•										*	
.)		_18030.000				0.000	1600.000	2240.000	1785.00		0.0	0.0	
-	. SR	425.000				0000	415.000	830.000	410.00			935.000	
	GR GR	404.000 405.000				5.000 3.000	399.600 415.000	1000.000 1130.000	400.70 420.00			1050.000 1265.000	
	<u> </u>					÷					•		
)	X1 GR	18620.000 421.300				0.000 3.000	560.000 414.300	520.000 772.000	590.00 411.60		0.0 00 409.000	0.0 795.000	

			٠.				•				•	
			·									ė.
	GR GR	404.600	862.000 -	405.400	883.000	401.700		400.000	937.000	402.700	9547000	
	NE	404.600 0.110	970,000 0,110	409,200	1000.000	411.000	1029.000	421.100	1100.000	0.0	0.0	
-		0.110	0.110	0.030	0.300	0.500	0.0	0.0	0.0	0.0	0.0	
										•		
<b>#</b> -	X1	20515.000	33.000	9638.000	9958.000	1720.000	1840.000	1895.000	0.0	0.0	0.0	
•	GR	420.900	8497,000	419.300	8534.000	413.900	8565.000	412,500	8639,000	413.800	8746.000	
	GR	411.600	8885.000	414.900	8896.000	416.100	8992.000	415.600	9021.000	410.700	9051.000	
	GR	411.000	9086.000	412.500	9103.000	410.000	9176.000	411.000	9213.000	411.400	9307.000	
	GR GR	411.900 405.300	9 <b>403.</b> 000 9681.000	412.100	9461.000	411.800	9513.000	412.300	9542+000	411,000	9638+000	4
	GR	402.000	9788.000	403.100 400.800	9699.000 9822.000	402,700	9716.000	403.900	9734.000	403.500	9741.000	
•	GR	408.400	9935.000	411.400	9958,000	401,300 423,100	9864.000 9987.000	402.000	9899.000	405.300	9921.000	
			. ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		7750.000	423.100	7707.000	0.0	0.0	0.0	0.0	
	XI	21425.000	38,000	9864.000	10000 000	700 000	1/00 000	040.000				
	ĜŔ	420+300	8700.000	417.400	10000.000 8712.000	700,000 413,400	1680.000 8727.000	910.000 414.600	0.0	0.0	0.0	
	GR	413.300	8872.000	413.200	8945.000	413,400	87271000	413.000	8792.000	415.300 417.700	8853.000 9131.000	
1	6R	417.700	9267.000	417.900	9363.000	416.700	9450.000	416.400	9043.000 9523.000	417.700	9610.000	
	GR	413,200	9643.000	414.900	9738.000	415.100	9833.000	413.200	9858.000	407,600	9864.000	•
	GR	405,600	9876.000	401.800	9889,000	400.400	9913.000	401.100	9931.000	402.200	9949.000	•
	GR	402,900	9962.000	405.600	9996.000	407.000	10000.000	408.700	10010.000	408,800	10025.000	
	GR	408.000	10032.000	412.200	10111.000	412.700	10160.000	416,100	10263.000	414.200	10275,000	
_	GR	418.000	10357.000	419.000	10365.000	423.000	10375.000	0.0	0.0	0.0	0.0	1
	X1	21965.000	21.000	848.000	1002.000	250,000	440.000	540.000	0.0	0.0	0.0	
	GR	419.700	375.000	417.300	450,000	415.200	483.000	416.000	545.000	416.100	593,000	
	GR	415.800	645.000	417.500	704.000	416.200	813.000	413.600	848.000	411,200	853.000	
4.0	GR GR	406.000 401.200	866.000	404.000 402.400	875.000	404+600	888.000	403.800	903.000	401.200	934,000	
	GR.	424.600	960.000 1015.000	0.0	973.000 0.0	406.000	988,000	411.800	1000.000	413.700	1002.000	
_	NC	0.100	0.110	0.035	0.300	0.0 0.500	0.0	0.0	0.0	0.0	0.0	
₽			Ÿ****	0.000	0.000	0.300		0.0	0.0	0.0	0.0	
Ė												
ω,	X1	23220,000	20.000	783.000	990.000	1000.000	880,000	1255.000	0.0	0.0	0.0	
	GR GR	419.800 403.500	776.000	415.600	783.000	408-100	796+000	407.000	799.000	404.700	819.000	
	GR	407.000	847.000 966.000	401.000	876.000	402.600	904.000	403,600	934.000	404.900	951.000	
	GR	414.500	1265.000	414.000 417.400	990.000 1335.000	414,400 415,200	1000.000 1438.000	414.500 413.200	1084.000	414.400	1183.000	
	٠.,	1211500	1203.000	417,400	13331000	413.200	1438.000	413+200	1468.000	419.800	1555.000	
• •	X1	25615.000	19.000	884.000	990.000	2400 000	720 000	2395.000		^ ^		•
	ĜŔ	420.900	871.000	410.000	884.000	2600,000 408,700	720.000 887.000	407.200	0.0 893.000	0.0 404.800	0.0 907.000	•
	SR	405.100	917.000	404.800	935.000	405.800	950.000	407.300	967.000	408.700	975.000	
	GR	410.300	990.000	411.200	1000.000	413.200	1055.000	413.200	1092.000	411.500	1105.000	
	GR	413.100	1159.000	415.700	1167.000	419.900	1185.000	420.900	1197.000	0.0	0.0	
	NC	0.085	0.110	0.040	0.300	0.500	0.0	0.0	0.0	0.0	0.0	
	X1	26545.000	31.000	884.000	992.000	600.000	480.000	930.000	0.0	0.0	0.0	
	'GR	434.700	752.000	415.400	788.000	415.400	811.000	414.200	830.000	414.600	847.000	•
	GR	412.900	886.000	411.300	889.000	408+200	900.000	408.800	918.000	409.800	933.000	
· 掩 -	GR	409.700	957.000	409.700	965,000	410.500	987+000	412.100	988.000	413.400	992.000	
•	GR	416.800	995.000	417.100	1000.000	419.000	1052.000	418.600	1066.000	417.300	1093.000	
	GR	416.900 423.200	1116.000 1291.000	420.100	1144.000	422.900	1201.000	421.600	1243.000	423.200	1283.000	. *
	GR	424.800	1683.000	420.400	1395,000	420.500	1531.000	419,900	1643.000	418.400	1656,000	
	NE	0.0	0.085	0.0	0.0 0.300	0.0 0.500	0.0	0.0	0.0 0.0	0.0	0.0	
<u>/</u>									and the second			
<u> </u>	X1	27595.000	14.000	855.000	1000-000	800.000	1320.000	1050 000	0.0	0.0		2.15
<u></u>	X1 GR	27595.000 440.600	14,000 816,000	855.000 440.200	1000.000	800.000 434.500	1320.000	1050.000	0.0 870.000	0.0 421.300	0.0	
, ,		27595.000 440.600 421.100	14.000 816.000 899.000	855.000 440.200 420.800	1000.000 848.000 918.000	800.000 434.500 421.700	1320.000 855.000 946.000	1050.000 423.500 421.600	0.0 870.000 <del>958</del> .000	0.0 421.300 <b>423.00</b> 0	0.0 890.000 971.000	

X1	27910.000	19.000	1000.000	1218.	000 3	265.000	350.000	315.000	0.0	0.0	0.0	
GR	449.200	868.000	447.600	872,		45.700	934.000	444,200	1000.000			
GR GR	428.500	1032.000	427.200	1042.	000	27.200						-
			427.200	10421	000	27+200	1051.000	427.800	1072.000			
GR	426.300	1095.000	425.800	1131.		126.500	1167.000	428.700	1181.000			
GR	439.000	1209.000	442.900	1218.		143.100	1233,000	449,200	1248.000	0.0	0.0	
NC	0.0	0.0	.0.0	0.	300	0.500	0.0	0.0	0.0	0.0	0.0	
	00044 000	17 300	1040 000	4454		05 055						
X1	28010.000	13.000	1048.000	1196.		85.000	130.000	100,000	0.0	0.0		
GR	453,300	980.000	452,400	1000,	000 4	144.400	1010.000	443,600	1048.000			- '
GR	430.800	1195.900	443.200	1196.		144 - 100	1208,000	446.500	1208.100	442.90	00 1216.000	
GR	439,800	1216,100	439.400	1228.	000 4	150.000	1228,100	0.0	0.0	0.0	-0.0	
SB	0.900	10.000	3.000	0.	0	0.100	0.010	0.010	0.0	434.40		
	<del>.</del>									•		
1 x1	28015.000	17.000	932.000	1076.	100	5.000	5.000	5.000	0.0	0.0	0.0	
F182	0,0	0.0	1.000	434.		140.400						
7135							0.0	0.0	0.0	0.0		
O E	17.000	864.000	453,300			84.000	452,400	0.0	894.000			
X2 BT BT	932.000	443.400	ó•o	932.		34.400	0.0	942.000	434.400			
181	443.400	0.0	944.000		400	0.0	944.100	440.400	0.0	1068.00	440.400	
# BT	0.0	1068,100	443.900			70.000	443,900	0.0	1070.100			
E BT	1076.000	436.900	0.0	1076.		143.900	0.0	1108.000	440.000			
BT BT BT BT	450.000	0.0	0.0	0.		0.0	0.0	0.0	0.0	0.0		
Z GR	453.300	864.000	452,400			144.400	894.000	443,400				
100	434.400								932.000			
<b>\4</b>		942,000	443,400	942.		43.400	944.000	440.400	944.100			
GR GR	443.900	1068.100	443,900	1070.		136.900	1070.100	436.900	1076.000			
3LGR	_ 440.000	1108,000	450,000	1108.		0.0	0.0	0.0	0.0	0.0		
-> NO	, 0.0	0.0	0.0	• 0.	300	0.500	.0.0	0.0	0.0	0.0		
L	ì				•							
×1	28027.000	17.000	1006,000	1173.	000	12.000	12.000	12.000	0.0	3.00	0.0	
GR	453.100	965.000	452,200	984.								
						149.600	1000.000	449.000	1006.000			
GR	440.800	1027.000	437.300	1035.		36.800	1063.000	433.800	1086.000			
GR	432.100	1126.000	435.300	1148.		134.800	1164.000	437.700	1167.000	444.60	00 1173,000	
GR	447,400	1173,100	448.100	1190.	000	0.0	0.0	0.0	0.0	0.0		
X1	28071.000	0.0	0.0	٥.	o	60.000	70.000	44.000	0.0	-3.00	0.0	
NC	0.080	0.0	0.035		300	0.500	0.0	0.0	0.0			
	0.000	0.0	0.033	٠,	300	0.300	0.0	0.0	0.0	0.0	, 0.0	
	00E0E 444	DD 4		445-					•			
X1	28525.000	25.000	1051.000	1179.	000 :	360.000	520.000	454,000	0.0	0.0	0.0	
GR	465.900	521.000	463.700	587.	000 4	162.800	619.000	461.500	666.000			
GR	455.800	815.000	451.700	858.	000	151 - 400	967,000	451.800	982.000			
GR	450.600	1003.000	441.800	1018.		441.600	1025.000	443.000	1033.000			
GR	440.900	1063.000	438.800		000	133.900	1094.000	436+200	1112.000		00 1140.000	,
GR	435.900	1164.000	440,900			45.000	1179.000	446,600	1200.000			
· EJ	0.0	0.0	0.0	0.		0.0						
	V+V	0.0	0.0	٠.	v	0+0	0.0	0.0	0.0	0.0	0.0	
	1717/1										•	
	1713/1 unn											
SUM	MARY PRINTO	in t			•							
		•					4					
<del>-</del>	SECNO	XLCH	ELTRD	ELLC	CWSEL	VI 8001	EL MEN	משכי	11617		0.05	
	020110	ALUIT	EFIND	LLLU	CWOEL	XLBEL	ELMIN	RBEL	VCH	Q	GLOB GCI	H QROB
	14790.000	220.00	0.0	0.0	400.97	405.20	394.50	405.10	1.96	1000.00	0.0 1000	.00 0.0
	14790.000	220,00	0.0	0.0	405.16	405.20	394.50	405.10	2.74	3000.00	0.0 3000	
	14790.000	220.00	0.0	0.0	407.03	405.20	394.50	405.10	3.22	4475,00	3.57 4468	
	14790.000	220.00										
			0.0	0.0	408.57	405,20		405.10		5875.00	31.26 5831	
	14790.000	220.00	0.0	0.0	409.65	405.20	394.50	405.10		7000.00	101.72 6872.	
	14790.000	220.00	0.0	0.0	410.51	405.20	394.50	405.10	3.99	8000.00	193.78 7765	
			0.0	0.0	411.35	405.20	394.50	405.10	4.15	9000.00	314.35 8624	16 61.50
	14790.000	220.00	0.0	***						, •		
·	14790.000							7.1				
· · · · ·		1455.00	0.0	0.0	401.32 405.51	402.00 402.00	394.30 394.30	402.00 402.00	1.43	1000.00	0.0 1000. 150.74 2753.	.00 0.0

2.6-

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 \$	16245.000 16245.000 16245.000 16245.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1455.00 1455.00 1455.00 1455.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0	408.99 410.08 410.97 411.82 403.14 406.54 408.32	402.00 402.00 402.00 402.00 404.00	394.30 394.30 394.30 394.30	402.00 402.00 402.00	2.44 2.57 2.67	5875.00 7000.00	518.24 725.22	4850.07 5585.07	506.69 689.72
 \$	16245.000 16245.000 16245.000 16245.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1455.00 1455.00 1455.00 1455.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0	408.99 410.08 410.97 411.82 403.14 406.54 408.32	402.00 402.00 402.00 402.00 404.00	394.30 394.30 394.30 394.30	402.00 402.00 402.00	2.44 2.57 2.67	5875.00 7000.00	518.24 725.22	4850.07 5585.07	506.69 689.72
 \$	16245.000 16245.000 16245.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1455.00 1455.00 1455.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0	410.08 410.97 411.82 403.14 406.54 408.32	402.00 402.00 402.00 404.00 404.00	394.30 394.30 394.30	402.00 402.00	2.57 2.67	7000.00	725.22	5585.07	689.72
 \$	16245.000 16245.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1455.00 1455.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	410.97 411.82 403.14 406.54 408.32	402.00 402.00 404.00 404.00	394.30 394.30	402.00	2.67				
<b>t</b>	18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1455.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	411.82 403.14 406.54 408.32	402.00 404.00 404.00	394.30			8000.00	074 /^		AE3 30
<b>t</b>	18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1785.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	403.14 406.54 408.32	404.00 404.00		702100			731+00	6210-62	857.79
•	18030.000 18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1785.00 1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0 0.0	0.0 0.0 0.0	406.54 408.32	404.00	399.60		2.76	9000.00	1159.11	6812.57	1028.32
•	18030.000 18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1785.00 1785.00 1785.00 1785.00 1785.00	0.0 0.0 0.0	0.0 0.0 0.0	408.32			404.00	3.70	1000.00	0.0	1000.00	0.0
•	18030.000 18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1785.00 1785.00 1785.00 1785.00 590.00	0.0 0.0	0.0		404 00	399.60	404.00	4.84	3000.00	30.28	2948.82	20.89
•	18030.000 18030.000 18030.000 18620.000 18620.000 18620.000	1785.00 1785.00 1785.00 590.00	0.0	0.0	409.80	404.00	399.60	404.00	5.49	4475.00	92.38	4315.66	66.96
	18030.000 18030.000 18620.000 18620.000 18620.000	1785.00 1785.00 590.00	0.0			404.00	399.60	404.00	5,97	5875.00	170.11	5577.41	127.48
	18030.000 18620.000 18620.000 18620.000	1785,00 590,00			410.84	404.00	399.60	404.00	6.31	7000.00	247.77	6561.96	190.27
	18620.000 18620.000 18620.000 18620.000	590.00	0.0	0.0	411-69	404.00	399.60	404.00	6.60	8000.00	328 <b>.9</b> 7	7416.08	254.95
	18620.000 18620.000 18620.000			0.0	412.50	404.00	399.60	404.00	6.84	9000.00	423.96	8247.28	328.76
	18620.000 18620.000		0.0	0.0	404.64	409.00	400.00	409.20	4.09	1000.00	0.0	1000.00	0.0
	18620,000	590.00	0.0	0.0	407.52	409.00	400.00	409.20	4,63	3000.00	0.0	3000.00	9.0
		590.00	0.0	0.0	409.23	409.00	400.00	409.20	4.60	4475.00	ο, ŏ3	4474.97	0.00
		590.00	0.0	0.0	410.66	409.00	400.00	409.20	4.63	5875.00	4.48	5863.09	7.43
٠	18620.000	590.00	0.0	0.0	411.71	409,00	400.00	409.20	4.70	7000.00	15.50	6952.74	31.76
•	18620.000	590,00	0.0	0.0	412.57	409.00	400.00	409.20	4.77	8000.00	32.57	7903.73	63.70
	18620.000	590,00	0.0	0.0	413.39	409.00	400.00	409.20	4.85	9000.00	52.65	8844.77	102.58
	20515.000	1895.00	0.0	0.0	40E E4	411 00	400 00					•	
	20515.000	1895.00	0.0	0.0	405.51	411.00	400.80	411,40	1.32	1000.00	0.0	1000.00	0.0
	20515.000	1895.00	0.0	0.0	408,46 410,12	411.00 411.00	400.80	411.40	1.97	3000.00	0.0	3000.00	0.0
	20515.000	1895.00	0.0	0.0	411.50	411.00	400.80 400.80	411.40	2.23	4475.00	0.01	4474.98	0.0
	20515.000	1895.00	0.0	0.0	412.50	411.00	400.80	411.40 411.40	2,40	5875.00	16.86	5858.14	0.00
	20515.000	1895.00	0.0	0.0	413.33	411.00	400.80	411.40	2.50 2.56	7000.00 8000.00	108.62	6871.23	0.15
	20515.000	1895.00	0.0	0.0	414.12	411.00	400.80	411.40	2,59	9000.00	280.23 518.85	7719.11 8479.54	0.66 1.60
								,,,,,,,	2,0,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	310.00	6477134	1+00
	SECNO	XLCH	ELTRD	ELLC	CWSEL	XLBEL	ELMIN	RBEL	VCH	Q	QLOB	QCH	QROB
	04.405.444								70		arob	QC/1	donu.
	21425.000 21425.000	910.00	0.0	0.0	405.79	407.60	400,40	407.00	2.50	1000.00	0.0	1000.00	0.0
	21425.000	910.00	0.0	0.0	408.71	407.60	400,40	407.00	3.82	3000.00	0.12	2996.22	3.66
-	21425.000	910.00 910.00	0.0	0.0	410.34	407.60	400.40	407.00	4+40	4475.00	1.28	4424.37	49.35
	21425.000	910.00	0.0	0.0	411.68	407.60	400.40	407.00	4.83	5875.00	3.64	5738.75	132.61
	21425.000	910,00	0.0	0.0	412.66	407.60	400.40	407.00	5.12	7000.00	10.33	6761.70	227.97
	21425.000	910.00	0.0	0.0	413.46 414.24	407,60 407,60	400.40	407.00	5.32	8000.00	40.98	7616.26	342.76
	, ,	, 20 7 4 4		0.0	717.24	407.60	400.40	407.00	5.46	9000.00	126.82	8395,26	477.92
	21965,000	540,00	0.0	0.0	406.09	413,60	401.20	413.70	2.61	1000.00	0.0	1000.00	0.0
	21965.000	540.00	0.0	0.0	409.04	413.60	401.20	413,70	3.92	3000.00	0.0	3000,00	0.0
	21965.000	540.00	0.0	0.0	410.65	413.60	401.20	413.70	4.52	4475.00	0.0	4475.00	0.0
,	21965.000	540.00	0.0	0.0	411.99	413.60	401.20	413.70	4.96	5875.00	0.0	5875+00	0.0
	21965.000	540.00	0.0	0.0	412.95	413.60	401.20	413.70	5.26	7000.00	0.0	7000.00	0.0
	21965.000	540,00	0.0	0.0	413.74	413.60	401.20	413.70	5.51	8000.00	0.01	7999.99	0.00
	21965.000	540.00	0.0	9.0	414.50	413.60	401.20	413.70	5.74	9000.00	1.07	8998.87	0.06
	23220.000	1255.00	0.0	0.0	406.77	415.60	401 00	444 00	1 00	1000 00			
	23220.000	1255.00	0.0	0.0	409.83	415.60	401.00	414.00	1.89	1000.00	0.0	1000.00	0.0
	23220.000	1255.00	0.0	0.0	411.50	415.60	401.00 4 <b>01.00</b>	414.00	2,82	3000.00	0.0	3000.00	0.0
	23220.000	1255.00	0.0	ŏ.ŏ	412.88	415.60	401.00	414.00 414.00	3.25 3.57	4475.00	0.0	4475.00	0.0
	23220.000	1255.00	0.0	0.0	413.87	415.60	401.00			5875.00	0.0	5875.00	0.0
<del></del>	23220.000	1253.00	0.0	0.0	414.68	415.60	401.00	414.00 414.00	3.80 3.97	7000.00 8000.00	0.0	6999 - 14	0.86
	23220.000	1255,00	0.0	0.0	415.45	415.60	401.00	414.00	4.10	9000.00	0.0	7985.40 8898.31	14.59 101.70
	· 00/45 000												
	25615.000	2395.00	0.0	0.0	409.15	410.00	404.80	410.30	3.56	1000.00	0.0	1000.00	0.0
	25615.000	2395.00	0.0	0.0	412.04	410.00	404.80	410.30	5.14	3000.00	1.23	2986 - 49	12.29
	25615.000 25615.000	2395,00	0.0	0.0	413.55	410.00	404.80	410.30	5.86	4475.00	5.21	4342,39	127.40
	25615.000	2395.00 2395.00	0.0	0.0	414.77	410.00	404.80	410,30	6.31	5875.00	11.09	5491.60	372.31
	25615.000	2375.00	0.0	0.0	415.67	410.00	404.80	410.30	6.60	7000.00	17,14	6371.37	611.49
	25615.000	2375.00	0.0	0.0	416.41 417.10	410.00 410.00	404.80	410.30	6.84	8000.00	23.36	7134.20	842.44
		•	***		417.10	410.00	404.80	410.30	7.06	9000.00	30.31	7884.79	1084.90
<del></del>	26545.000	930.00		0.0	411.90	412.90	408.20	413.40	4.28	1000,00			

														<b>- -</b> .
	26545.000	930.00	0.0	0.0	414,35	412,90	408.20	413.40	6.11	3000.00	9.16	2990.70	0.15	
	26545.000	930.00	0.0	0.0	415.65	412.90	408.20	413.40	6.94	4475.00	123.76	4349.81	1.43	
<del></del>	26545.000	930.00	0.0	0.0	416,69	412.90	408.20	413.40	7.51	5875.00	341.56	5529.61		
1.	26545.000	930.00	0.0	0.0	417.46	412,90	408.20	413.40	7.86	7000.00			3.83	
	26545+000	930.00	0.0	0.0	418.10	412,90					555.05	6432.73	12.22	
4	26545.000						408.20	413.40	8,12	8000.00	759.47	7196.70	43.83	
-	20343+000	930.00	0.0	0.0	418.71	412,90	408.20	413.40	8,33	9000.00	971.74	7925.23	103.03	
	27595.000	1050.00	0.0	0.0	423.10	434,50	420.80	434.20	6.56	1000.00	0.0	1000.00	0.0	-
	27595.000	1050.00	0.0	0.0	424,61	434.50	420.80	434.20	9.66	3000.00	0.0	3000.00	0.0	, '
*	27595.000	1050.00	0.0	0.0	425,50	434,50	420.80	434.20	10.95	4475.00	0.0	4475.00	0.0	
*	27595.000	1050.00	0.0	0.0	426,26	434,50	420.80	434.20	11.88	5875.00	0.0	5875.00	0.0	
_ · *	27595.000	1050.00	0.0	0.0	426,83	434,50	420.80	434.20	12.51	7000.00	0.0	7000.00		
*		1050.00	ŏ.ŏ	0.0	427,29	434,50	420.80	434.20	13.02	8000.00	ŏ.ŏ	8000.00	0.0	
*	27595.000	1050.00	0.0	0.0	427.74	434.50	420.80	434.20	13.47	9000.00	0.0	9000.00	0.0	
	27910.000	315.00	0.0		***					•				
				0.0	428,07	444,20	425.80	442.90	5.17	1000.00	0.0	1000.00	0.0	
4-	27910.000	315.00	0.0	0.0	429,52	444.20	425.80	442.90	7.36	3000.00	0.0	3000.00	0.0	
•	27910.000	315.00	0.0	0.0	430.34	444.20	425.80	442.90	8.35	4475.00	0.0	4475.00	0.0	
	27910.000	315.00	0.0	0.0	431,06	444.20	425.80	442.90	9+04	5875.00	0.0	5875.00	0.0	
	27910.000	315.00	0.0	0.0	431,60	444,20	425.80	442.90	9.48	7000.00	0.0	7000.00	0.0	
	27910.000	315.00	0.0	0.0	432.08	444,20	425.80	442.90	9,80	8000.00	0.0	8000.00	0.0	
	27910.000	315.00	0.0	0.0	432,53	444.20	425.80	442.90	10.10	9000.00	0.0	9000.00	0.0	
											•			
	SECNO	XLCH	ELTRD	ELLC	CWSEL	XLBEL	ELMIN	RBEL	VCH	O O	QLOB	асн	ORDB	
*	28010.000	100.00	0.0	0.0	431,92	443.60	430.80	443.20	6.05	1000.00	0.0	1000.00	0.0	
*		100.00	0.0	0.0	433.13	443.60	430.80	443.20	8.70	3000.00	0.0	3000.00	ŏ.ŏ	
*	28010.000	100.00	0.0	0.0	433.85	443.60	430.80	443.20	9.94	4475.00	0.0	4475.00	0.0	
4 ×		100.00	0.0	0.0	434.45	443.60	430.80	443.20	10.90	5875.00	0.0	5875.00	0.0	
*		100.00	0.0	0.0	434.90	443,60	430.80	443.20	11.55	7000.00	0.0			
.4.		100.00	0.0	0.0	435.30	443.60						7000.00	0.0	
*		100.00	0.0	0.0	435.66	443.60	430.80 430.80	443.20 443.20	12.04 12.53	8000.00 9000.00	0.0	9000.00	0.0	
* ;*													V+V	
	28015.000	5.00	440.40	434.40	440,99	443.40	434.40	443.90	6.11	1000.00	0.0	992.92	7.07	
₽6*	28015.000	5.00	440.40	434 • 40	442,29	443.40	434.40	443.90	8.52	3000.00	0.0	2941.63	58.37	
52.	28015.000	5.00	440.40	434.40	443,00	443,40	434.40	443.90	7.80	4475.00	0.0	4356.69	118.31	
75.4*	28015.000	5.00	440.40	434.40	443.59	443,40	434.40	443.90	10.76	5875.00	0.37	5683.52	191.12	
[ <b>*</b> ]	28015.000	5.00	440.40	434.40	444.15	443,40	434.40	443.90	11.05	7000.00	12.73	6713.12	274,15	
⊙*	28015.000	5.00	440.40	434.40	444.49	443.40	434+40	443.90	11.60	8000.00	36.25	7616.17	347.58	
* <del>*</del> *	28015.000	5.00	440.40	434,40	444.94	443,40	434.40	443.90	11.74	9000.00	89.44	8476+27	434.28	
• 6	28027.000	12.00	0.0	0.0	441.66	452,00	435.10	447.60	2.03	1000.00	0.0	1000.00	0.0	
	28027.000	12.00	0.0	0.0	443,43	452.00	435.10	447.60	4.07	3000.00	0.0	3000.00		
	28027.000	12.00	0.0	0.0	444.40	452.00	435.10	447.60	5.10				0.0	
	28027.000	12.00	0.0	0.0	445.20	452.00				4475.00	0.0	4475.00	0.0	
	28027.000	12.00					435.10	447.60	5.92	5875.00	0.0	5875.00	0.0	
			0.0	0.0	445.70	452.00	435.10	447.60	6.57	7000.00	0.0	7000.00	0.0	
	28027,000	12.00	0.0	0.0	446.13	452.00	435.10	447.60	7.08	8000.00	0.0	8000.00	0.0	
	28027.000	12.00	0.0	0.0	446,47	452.00	435.10	447.60	7.63	9000.00	0.0	9000.00	0.0	
<u> </u>	28071.000	44.00	0.0	0.0	441.73	449.00	432.10	444.60	1.08	1000.00	0.0	1000.00	0.0	
	28071.000	44.00	0.0	0.0	443.66	449.00	432.10	444.60	2,48	3000.00	0.0	3000.00	ŏ.ŏ	
* * <u>1</u>	28071.000	44.00	0.0	0.0	444,75	449.00	432.10	444.60	3.25	4475.00	0.0	4475.00	0.00	
<b>4</b>	28071.000	44.00	0.0	0.0	445.64	449.00	432.10	444.60	3.23	5875.00	0.0	5875.00	0.00	
	28071.000	44.00	0.0	0.0	446.24	449.00	432.10	444.60	4.34	7000.00	0.0	7000.00	0.00	
	28071.000	44.00	0.0											
	28071.000			0.0	446.74	449.00	432.10	444.60	4.73	8000.00	0.0	8000,00	0.00	
-	200/1:000	44.00	0.0	0.0	447.17	449.00	432.10	444.60	5.11	9000.00	0.0	8999•99	0.01	
<u></u>	28525.000	454.00	0.0	0.0	441.77	443.20	433.90	445.00	1.59	1000.00	0.02	999.98	0.0	
	28525.000	454.00	0.0	0.0	443.80	443,20	433.90	445.00	3.40	3000.00	21.85	2978 - 15	0.0	
	28525.000	454.00	0.0	0.0	444,94	443.20	433.90	445.00	4.31	4475.00	71.94	4403.05	0.0	
	28525.000	454.00	0.0	0.0	445.67	443.20	433.90	445.00	5.03	5875.00	136.07	5737.52	1.40	
<del></del> -	28525.000	454.00	0.0	0.0	446.50	443.20	433.90	445.00	5.57	7000.00	193.56	6800.14	6.30	
	28525.000	454.00	0.0	0.0	447.03	443.20	433.90	445.00	6.00	8000.00	249.93	7734.07	16.00	
•	28525.000	454.00												
	200201000		0.0	0.0	447.48	443,20	433.90	445.00	6.43	9000+00	307.57	8664.00	28.42	

- 7

## APPENDIX E

INFORMATION AS CONTAINED IN THE NATIONAL INVENTORY OF DAMS

## 限器 INVENTORY OF DAMS IN THE UNITED STATES

	A NAME OF	<u> </u>				• • • • • • • • • • • • • • • • • • • •		• • • • • • • • • • • • • • • • • • • •								
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				POPU	LAR 1	AME.		Ì		NAME OF	IMPOUNDMENT	Г		•		
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